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CONSTRUCTION PLANS FOR  
FRUITVALE ISD  
ADMINISTRATION BUILDING  
FRUITVALE, TEXAS  
PHASE 2



Fruitvale  
INDEPENDENT SCHOOL DISTRICT

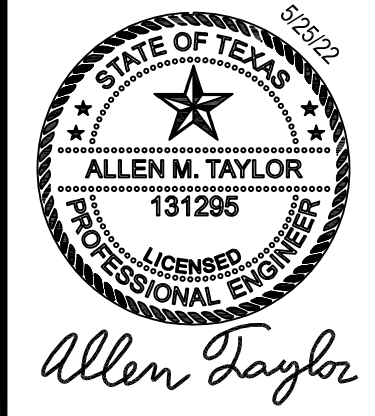


CITY OF  
FRUITVALE, TX

LOCATION MAP  
NOT TO SCALE

PROJECT INFORMATION	
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<b>OWNER</b> FRUITVALE ISD ADMINISTRATION OFFICE P. O. BOX 77 FRUITVALE, TEXAS 75127 PHONE: 903.896.4729 FAX: 903.896.1011	<b>STRUCTURAL ENGINEER</b> EXCEED ENGINEERING CONTACT: ALLEN M. TAYLOR, P.E. PHONE: 903.258.9987
	<b>CIVIL ENGINEER</b> EXCEED ENGINEERING CONTACT: ALLEN M. TAYLOR, P.E. PHONE: 903.258.9987
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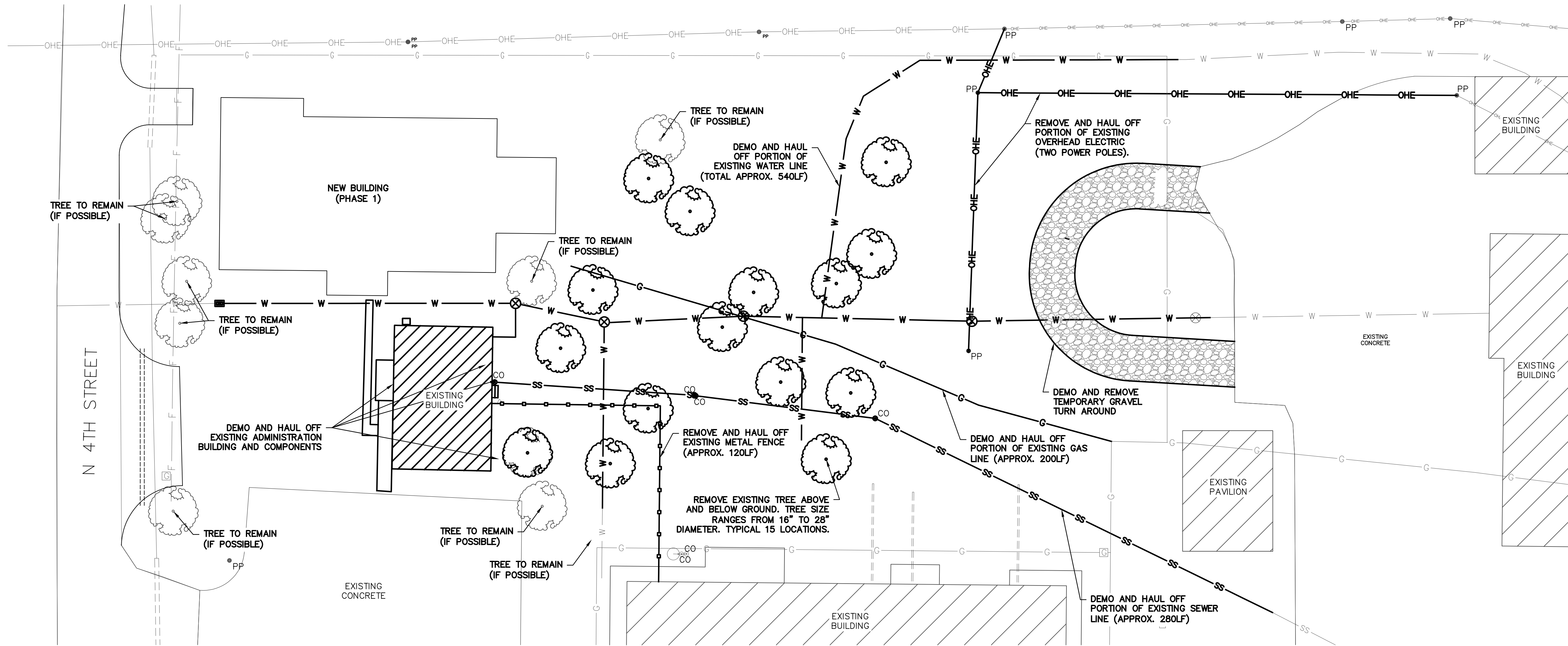


2021 FRUITVALE ISD  
ADMINISTRATION BLDG  
FRUITVALE, TEXAS 75127  
COVER SHEET (PHASE 2)



Issue/Revision	Date & Description
IFC	5-25-22
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C0	

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DEMOLITION PLAN

20 10 0 20 40 60

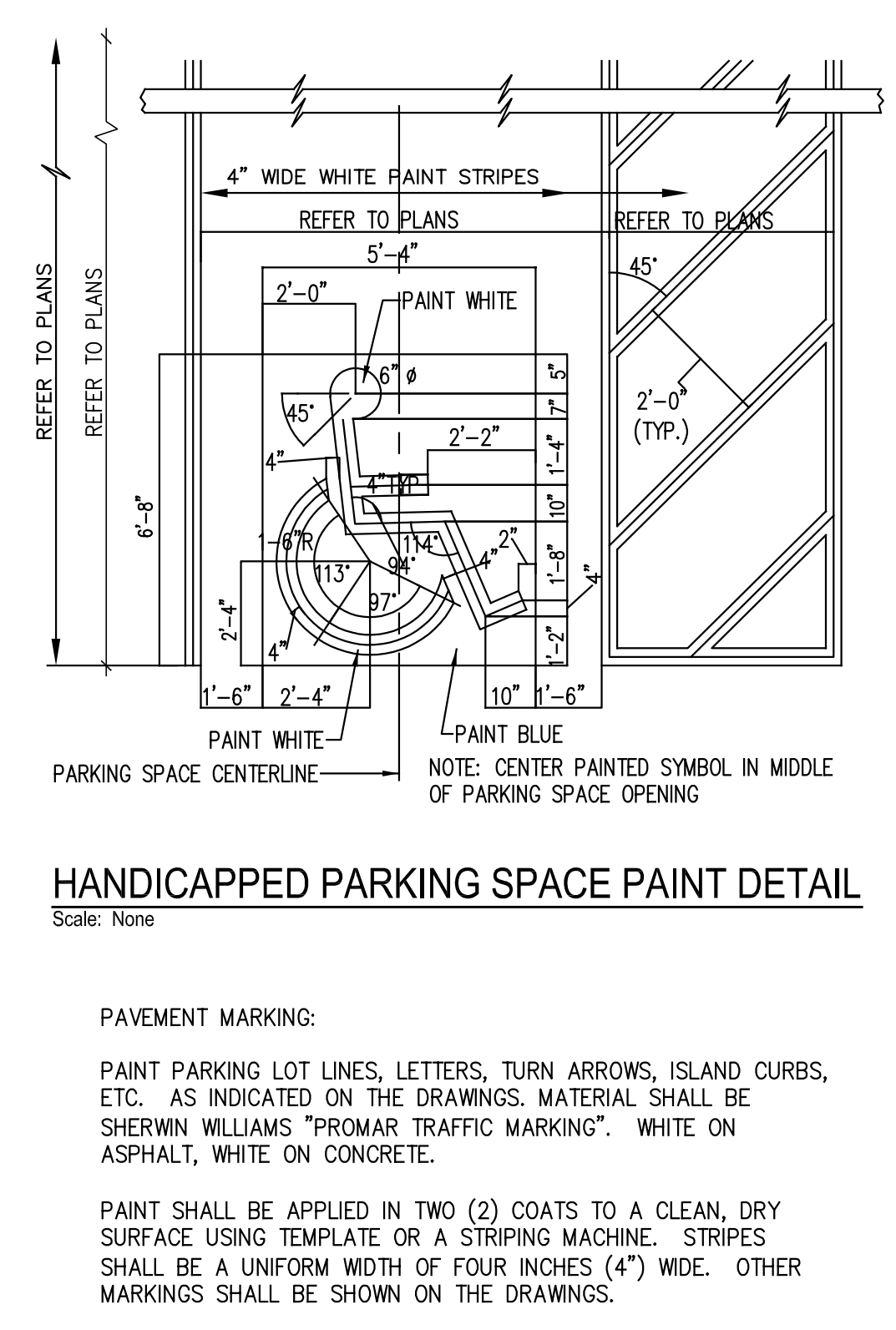
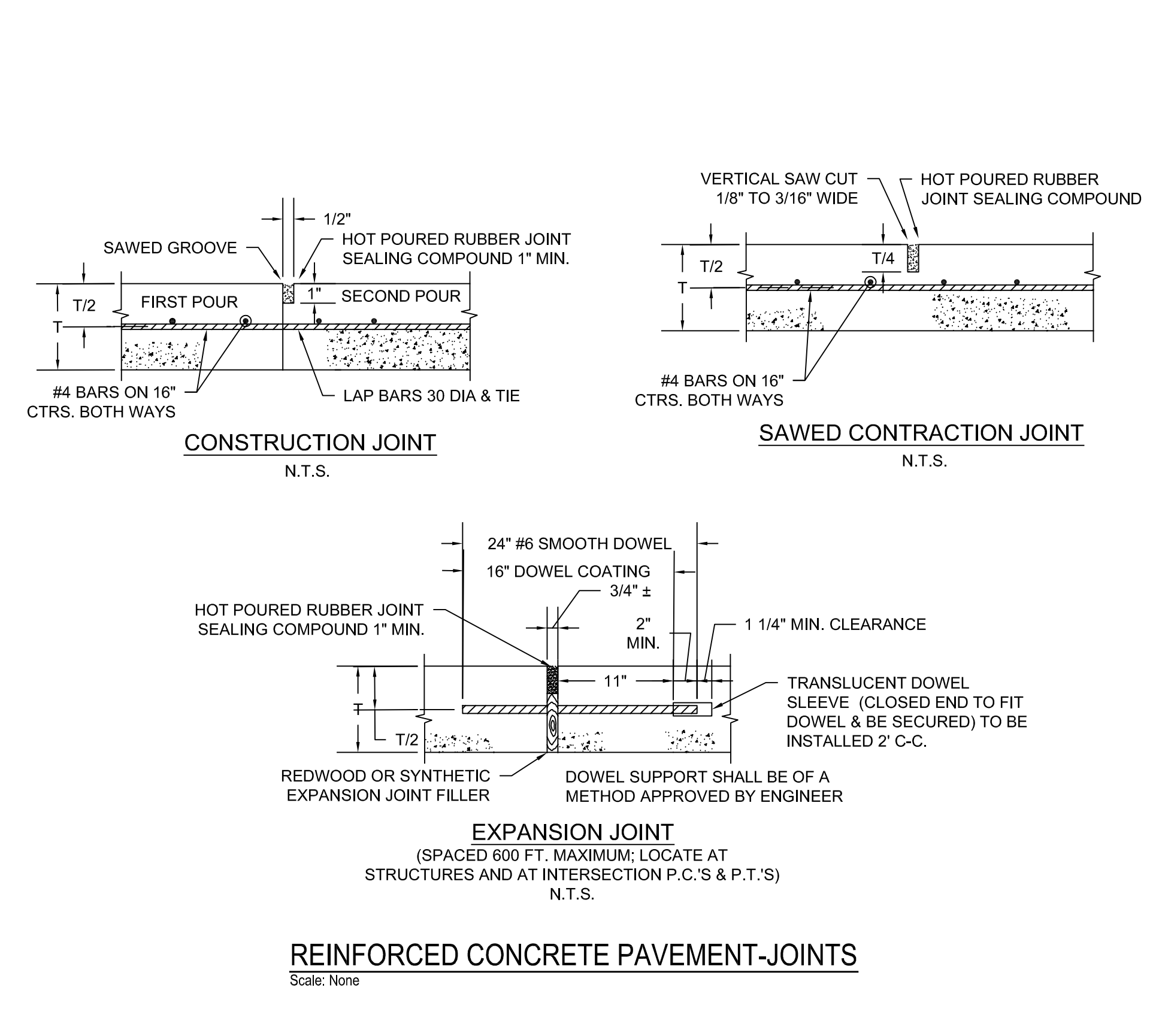
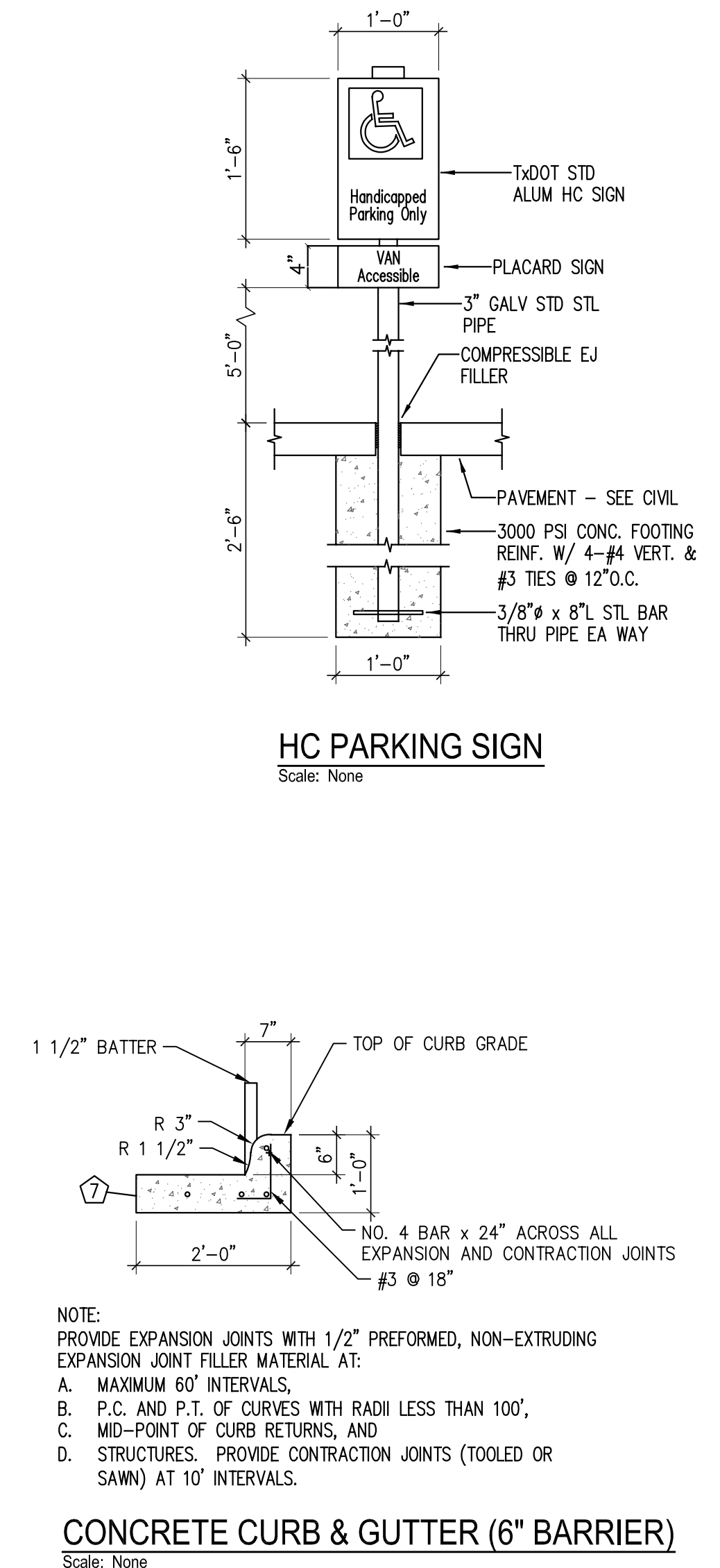
SCALE : 1" = 20'

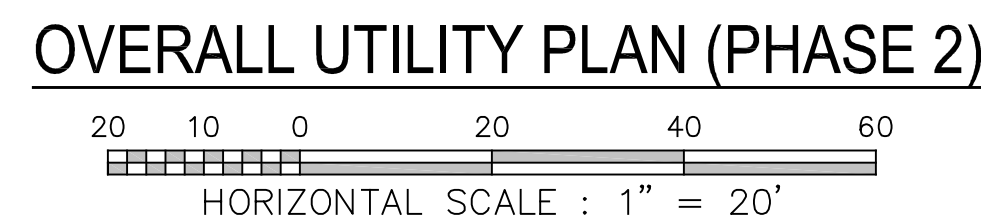
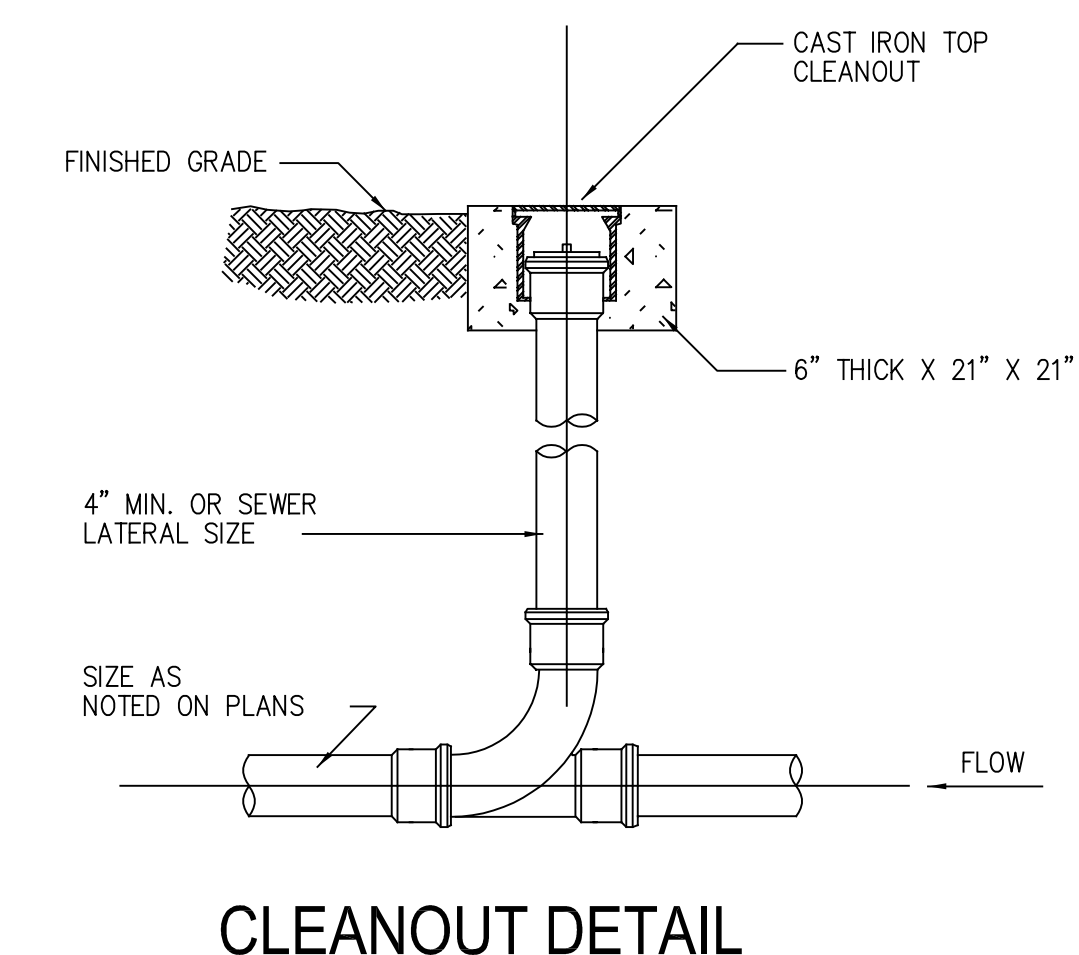
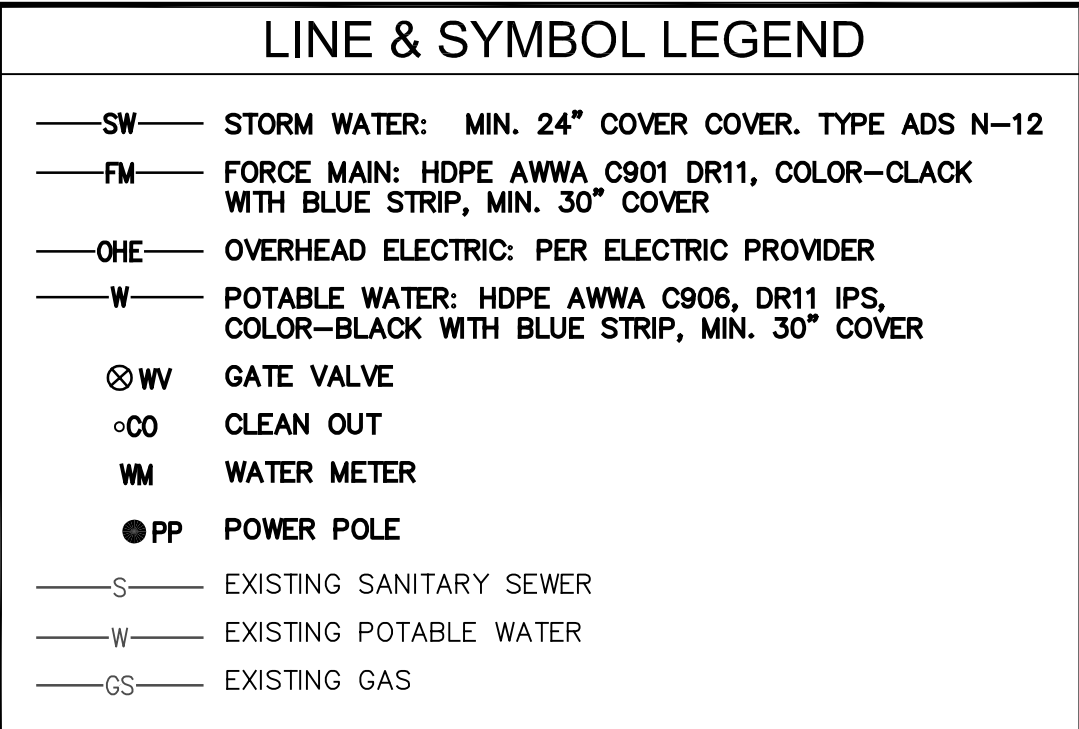
## GENERAL NOTES

- THESE DRAWINGS ARE ISSUED TO INDICATE THE GENERAL SCOPE OF THE PROJECT.
- CONTRACTOR IS TO FURNISH ALL ITEMS REQUIRED FOR PROPER COMPLETION OF THE WORK WITHOUT ADJUSTMENT TO CONTRACT PRICE. COMPLETED WORK SHALL BE OF SOUND AND QUALITY CONSTRUCTION AND THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE INCLUSION OF ADEQUATE AMOUNTS TO COVER INSTALLATION OF ALL ITEMS INDICATED.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING AND SHORING REQUIRED FOR COMPLETION OF THE WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS EMPLOYED FOR THE COMPLETION OF THE PROJECT.
- THE CONTRACTOR SHALL OBTAIN ALL PERMITS NECESSARY FOR COMPLETION OF THE WORK.
- THE CONTRACTOR SHALL ARRANGE ALL REQUIRED INSPECTIONS.
- THE CONTRACTOR SHALL VERIFY THE LOCATION, DEPTH, AND SIZE OF EXISTING UTILITIES WHETHER SHOWN ON THE PLANS OR NOT BEFORE BEGINNING CONSTRUCTION.
- THE INFORMATION SHOWN ON THESE DRAWINGS CONCERNING TYPE AND LOCATION OF UNDERGROUND AND OTHER UTILITIES IS NOT GUARANTEED TO BE ACCURATE OR ALL-INCLUSIVE. THE CONTRACTOR IS RESPONSIBLE FOR MAKING HIS OWN DETERMINATIONS AS TO THE TYPE AND LOCATION OF ALL UNDERGROUND AND OTHER UTILITIES AS MAY BE NECESSARY IN AVOIDING DAMAGE THERETO. AS REQUIRED BY "THE TEXAS UNDERGROUND FACILITY DAMAGE PREVENTION AND SAFETY ACT" TEXAS ONE CALL SYSTEM MUST BE CONTACTED AT LEAST 48 HOURS PRIOR TO ANY EXCAVATION OPERATIONS BEING PERFORMED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CONTACT TEXAS ONE CALL SYSTEM.
- THE CONTRACTOR SHALL REPAIR ALL PORTIONS OF PAVEMENT DAMAGED BY HIS EQUIPMENT, AT CONTRACTOR'S EXPENSE.
- ALL AREAS DISTURBED BY CONSTRUCTION SHALL BE RESTORED TO THEIR ORIGINAL OR BETTER CONDITION, UNLESS OTHERWISE NOTED.
- ALL BACKFILL SHALL BE PLACED IN ACCORDANCE WITH THE TYPICAL TRENCH DETAIL SHOWN ON MISC. DETAIL SHEET.
- EXISTING IMPROVEMENTS INCLUDING, BUT NOT LIMITED TO FENCES, DRIVEWAYS, SIDEWALKS, PAVEMENT, CURBS, UTILITY AND OIL OR GAS PIPELINES, DRAINAGE STRUCTURES WHICH ARE REMOVED OR ALTERED TO PERMIT INSTALLATION OF THE WORK SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR, AT THE CONTRACTOR'S EXPENSE, IN THE SAME LOCATION AND IN A CONDITION AS GOOD OR BETTER THAN FOUND.
- CONTRACTOR SHALL TAKE SPECIAL PRECAUTIONS IN THE VICINITY OF ANY OVERHEAD ELECTRIC LINES. CONTRACTOR SHALL ABIDE BY NATIONAL ELECTRIC CODE AND ANY REQUIREMENT BY OWNER OF THE ELECTRIC LINE.
- CONTRACTOR SHALL NOTIFY OWNER AND RECEIVE AUTHORIZATION FOR THE DATE AND TIME FOR WHICH THE PROPOSED UTILITIES WILL BE CONNECTED TO THE EXISTING UTILITIES.
- CONTRACTOR SHALL MAINTAIN ONE SET OF "AS-BUILT" DRAWINGS SHOWING CLEARLY ALL FIELD CONDITIONS, WHETHER EXISTING OR PROPOSED, WHICH DIFFER FROM THE CONDITIONS SHOWN ON THESE DRAWINGS. THE LOCATION, SIZE AND TYPE OF ALL UTILITY AND PIPELINES ENCOUNTERED THROUGHOUT THE PROJECT SHALL BE SHOWN ON THE "AS-BUILT" DRAWINGS.
- FOR UTILITY LINES UNDER PAVED AREAS, TRENCH SHALL BE BACKFILLED AND COMPACTED IN 8" LOOSE LIFTS WITH SELECT MATERIAL. NONPLASTIC MATERIAL SHALL BE CEMENT STABILIZED. COMPACTION OF CLAY MATERIAL SHALL BE BY HAND-HELD MECHANICAL TAMPER. ("WACKER-PACKER" OR EQUAL) TO 95% STANDARD PROCTOR DENSITY.
- THE CONTRACTOR SHALL INSTALL AND MAINTAIN SILT FENCING AS REQUIRED BY TXDOT, TCEQ, & EPA.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED TO COMPLETE THE WORK.
- NO FIRES WILL BE ALLOWED UNLESS OTHERWISE NOTED.
- ALL DRIVEWAYS THAT ARE DAMAGED AS A RESULT OF THE CONSTRUCTION SHALL BE REPAIRED TO A CONDITION EQUAL TO OR BETTER THAN THEIR CONDITION PRIOR TO THE CONSTRUCTION, USING LIKE MATERIALS. ALL CONCRETE DRIVEWAYS SHALL BE REPAIRED WITH CONCRETE, ALL OTHERS SHALL BE REPAIRED WITH ASPHALT.
- ADDITIONAL UTILITY AND PIPELINES NOT SHOWN ON THE DRAWINGS MAY BE ENCOUNTERED THROUGHOUT THE PROJECT. EXACT LOCATIONS OF ALL EXISTING UNDERGROUND UTILITIES AND PIPELINES, WHETHER SHOWN ON THESE PLANS OR NOT, SHOULD BE DETERMINED IN THE FIELD PRIOR TO EXCAVATION.
- NO REVISIONS SHALL BE MADE IN THESE PLANS WITHOUT THE APPROVAL OF THE DESIGN ENGINEER OR AN AUTHORIZED REPRESENTATIVE OF THE OWNER.
- THE CONTRACTOR IS RESPONSIBLE FOR JOBSITE CONDITIONS AT ALL TIMES DURING CONSTRUCTION AND SHALL DEFEND, INDEMNIFY AND HOLD HARMLESS THE OWNER, HIS AGENTS AND THE ENGINEER AGAINST ANY CLAIMS RESULTING FROM SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION. THE JOBSITE SHALL BE MAINTAINED IN A SAFE CONDITION DURING AND AFTER WORK HOURS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
- CONTRACTOR SHALL PROVIDE AND MAINTAIN PROTECTIVE FENCING TO SECURE THE ENTIRE CONSTRUCTION SITE PRIOR TO DEMOLITION FOR THE DURATION OF THE PROJECT.

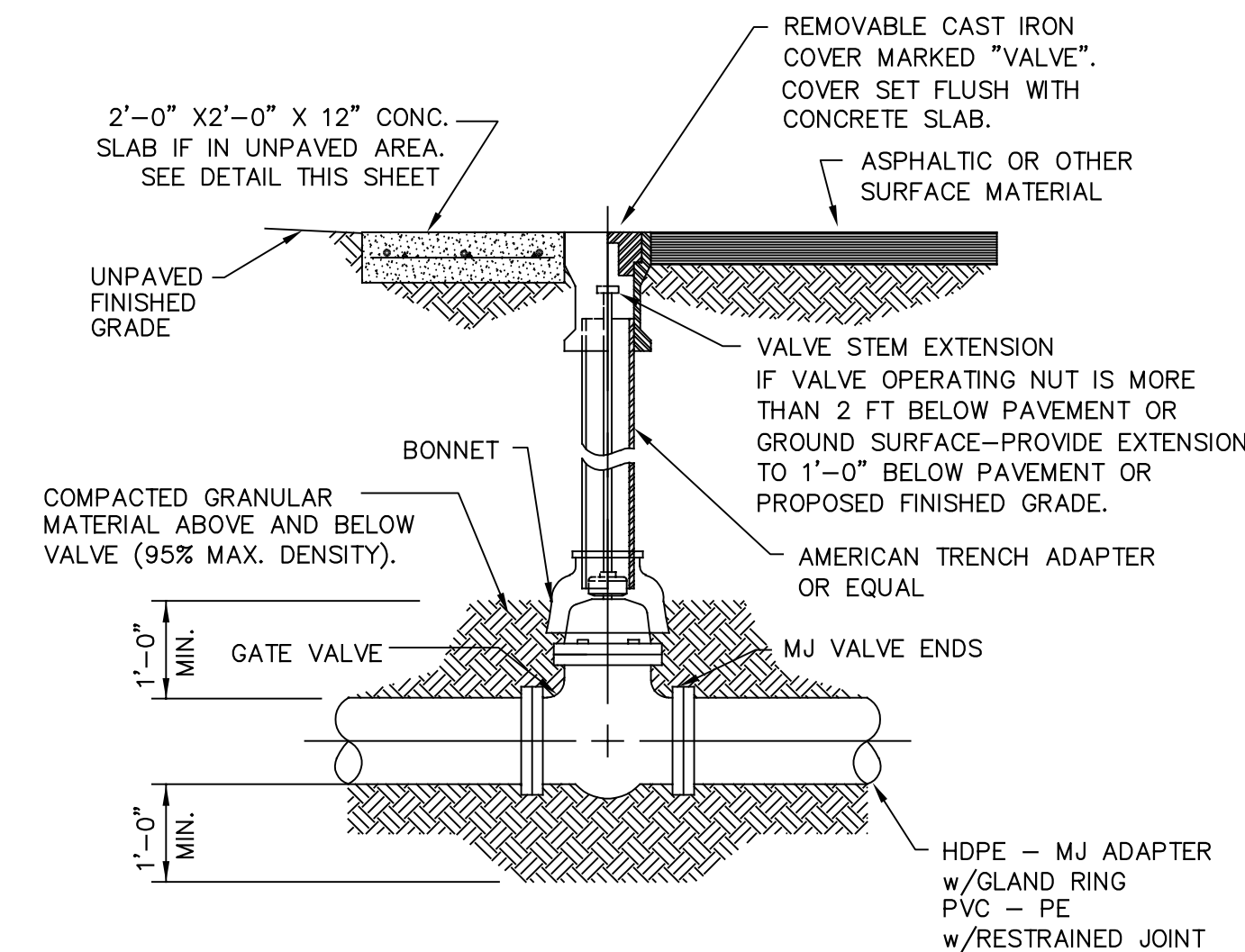
## CAUTION EXISTING UTILITIES

EXISTING UTILITIES AND UNDERGROUND FACILITIES INDICATED ON THESE PLANS HAVE BEEN LOCATED FROM REFERENCED INFORMATION. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY BOTH HORIZONTAL AND VERTICAL LOCATIONS OF ALL EXISTING UTILITIES AND UNDERGROUND FACILITIES PRIOR TO CONSTRUCTION AND TO TAKE NECESSARY PRECAUTIONS TO PROTECT ALL FACILITIES ENCOUNTERED. THE CONTRACTOR SHALL PRESERVE AND PROTECT ALL EXISTING UTILITIES FROM DAMAGE DURING CONSTRUCTION.





1. TRENCH ALIGNMENT SHALL BE AS STRAIGHT AS CONDITIONS PERMIT. ANY DEVIATIONS FROM PLANNED ALIGNMENT SHALL HAVE PRIOR APPROVAL BY THE PROJECT ENGINEER/INSPECTOR. ALL TRENCH CUTS SHALL BE IN ACCORDANCE WITH EXISTING SAFETY REGULATION IN EFFECT
2. TRENCH BOTTOM SHOULD BE UNDISTURBED, TAMPED, OR RELATIVELY SMOOTH EARTH. WHERE EXCAVATION IS IN ROCK, THE CONDUIT SHOULD BE LAID ON A LAYER OF CLEAN BACKFILL.
3. ALL BACKFILL SHOULD BE FREE OF DEBRIS OR OTHER MATERIAL THAT MAY DAMAGE THE CONDUIT SYSTEM. THE BACKFILL MATERIAL SHOULD FILL THE VOIDS AROUND THE CONDUIT TO PREVENT HOT SPOTS & SETTLING.
4. BACKFILL SHOULD BE ADEQUATELY COMPACTED. BACKFILL NOT UNDER PAVEMENT SHOULD BE COMPACTED TO THE SAME DENSITY AS THE SURROUNDING UNDISTURBED BACKFILL. UNDER PAVEMENT SHOULD BE COMPACTED TO NOT LESS THAN 95% OF THE DENSITY OF UNDISTURBED SOIL AS DETERMINED BY ASTM D-698.



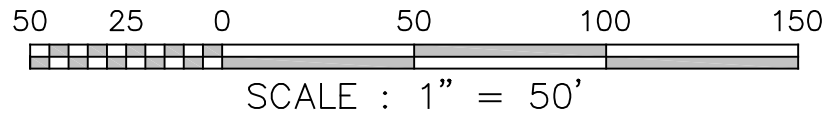
**NOTES:**

1. ALL BURIED VALVES SHALL BE PROVIDED W/EXTENSION STEM OPERATOR W/ 2" SQ. AWWA NUT WITHIN 1' BELOW FINISHED GRADE. NUT IS TO INDICATE DIRECTION OF ROTATION TO OPEN VALVE.
2. COAT BURIED PIPE & VALVE BOX PER SPECIFICATIONS. WRAP WITH 8 MIL. POLYETHYLENE.
3. CLEAN VALVE BOX OF ALL DEBRIS AND SOIL.
4. VALVE SIZE & TYPE AS SHOWN ON PLANS.
5. MUELLER 2300 RESILIENT WEDGE GATE VALVE OR APPROVED EQUAL.

### TYPICAL VALVE, VALVE BOX AND EXTENSION INSTALLATION DETAIL

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PRE-DEVELOPED DRAINAGE CALCULATIONS (PHASE 2)



SN	Element	Area	Drainage	Weighted	Accumulated	Total	Peak	Rainfall	Time of
	ID	(acres)	Node ID	Runoff	Precipitation	Runoff	Runoff	Intensity	Concentration
				Coefficient	(inches)	(inches)	(cfs)	(inches/hr)	(days hh:mm:ss)
1	Sub-01	1.77	Out-01	0.5200	2.99	1.56	3.32	3.611	0 00:49:36

CALCULATIONS BASED OFF OF 25YR STORM USING RATIONAL METHOD.

Issue/Revision Date & Description

IFC: 5-25-22

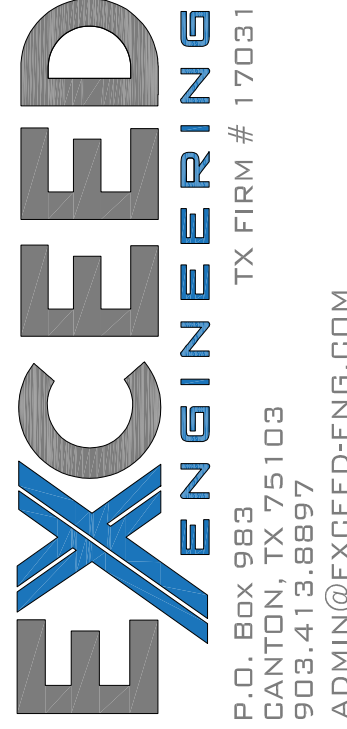
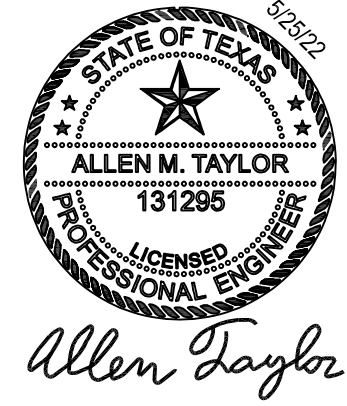
Job Number:  
050-21003

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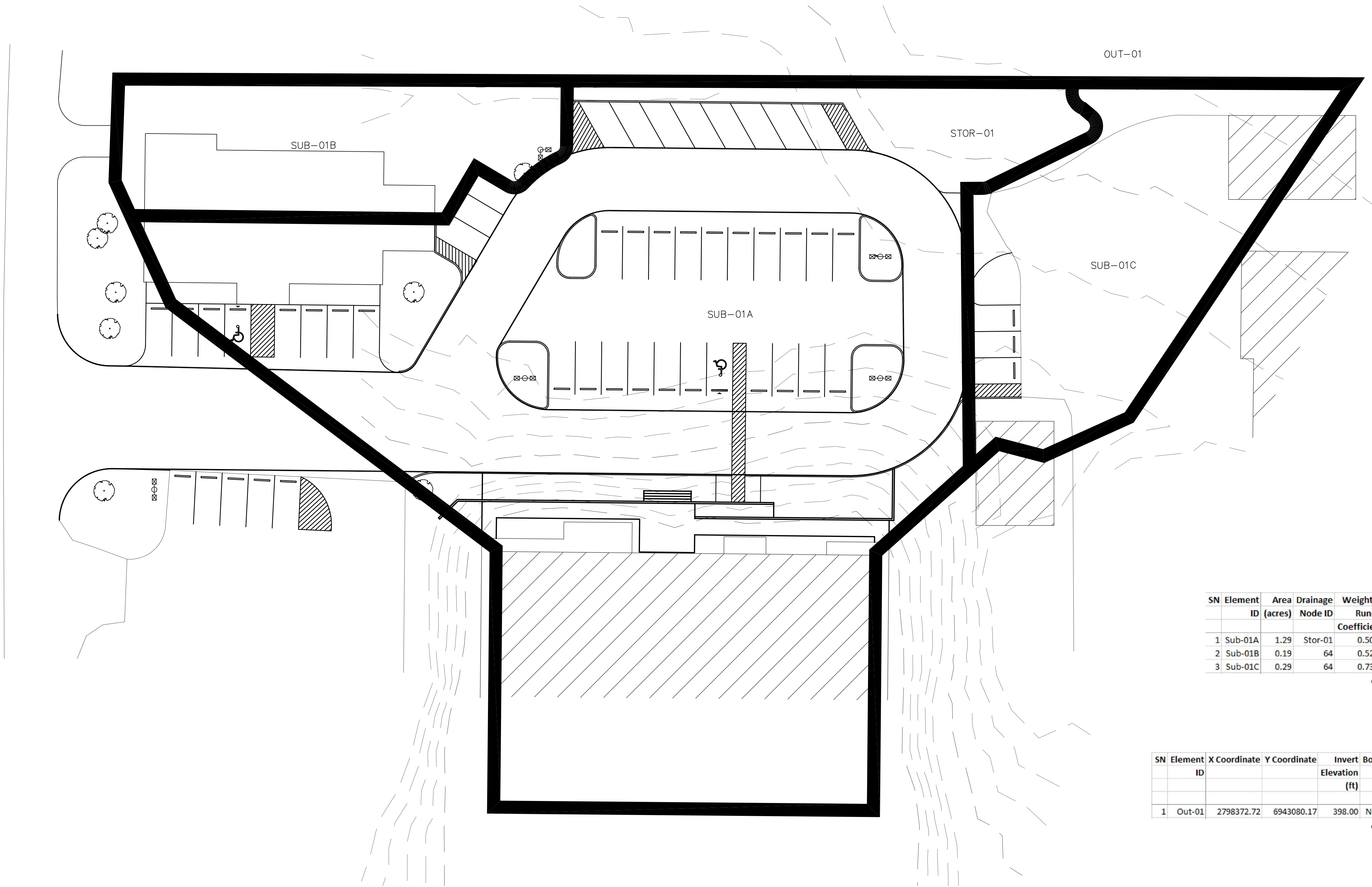
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2021 FRUITVALE ISD  
ADMINISTRATION BLDG  
FRUITVALE, TEXAS 75127  
PRE-DEVELOPED DRAINAGE CALCULATION (PHASE 2)



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SN	Element ID	Area (acres)	Drainage Node ID	Weighted Runoff Coefficient	Accumulated Precipitation (inches)	Total Runoff (inches)	Peak Runoff (cfs)	Rainfall Intensity (inches/hr)	Time of Concentration (days hh:mm:ss)
1	Sub-01A	1.29	Stor-01	0.5000	1.31	0.65	5.06	7.848	0 00:10:00
2	Sub-01B	0.19	64	0.5200	3.05	1.59	0.35	3.494	0 00:52:24
3	Sub-01C	0.29	64	0.7300	1.31	0.96	1.66	7.848	0 00:10:00

CALCULATIONS BASED OFF OF 25YR STORM USING RATIONAL METHOD.

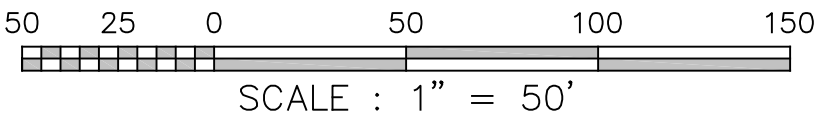
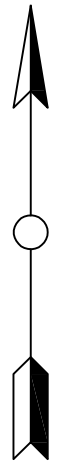
SN	Element ID	X Coordinate	Y Coordinate	Invert Elevation (ft)	Boundary Type	Flap Gate	Fixed Water Elevation (ft)	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Maximum HGL Depth (ft)	Maximum HGL Elevation (ft)
1	Out-01	2798372.72	6943080.17	398.00	NORMAL	NO		2.77	0.00	0.32	398.32

CALCULATIONS BASED OFF OF 25YR STORM USING RATIONAL METHOD.

SN	Element ID	X Coordinate	Y Coordinate	Invert Elevation (ft)	Max (Rim) Elevation (ft)	Max (Rim) Offset (ft)	Initial Water Elevation (ft)	Initial Water Depth (ft)	Ponded Area (ft²)	Evaporation Loss	Peak Inflow (cfs)	Peak Lateral Inflow (cfs)	Peak Outflow (cfs)	Peak Exfiltration Flow Rate (cfm)	Max HGL Elevation (ft)	Max HGL Depth (ft)	Average HGL Elevation (ft)	Average HGL Depth (ft)	Time of Occurrence (days hh:mm)	Total Max HGL Exfiltration Volume (1000-ft³)	Total Flooded Volume (ac-inches)	Total Time Flooded (minutes)	Total Retention Time (seconds)
1	Stor-01	2798438.07	6943031.99	398.80	400.50	1.70	0.00	-398.80	0.00	0.00	5.06	5.06	1.25	0.00	400.25	1.45	398.87	0.07	0 00:17	0.00	0.00	0.00	0.00

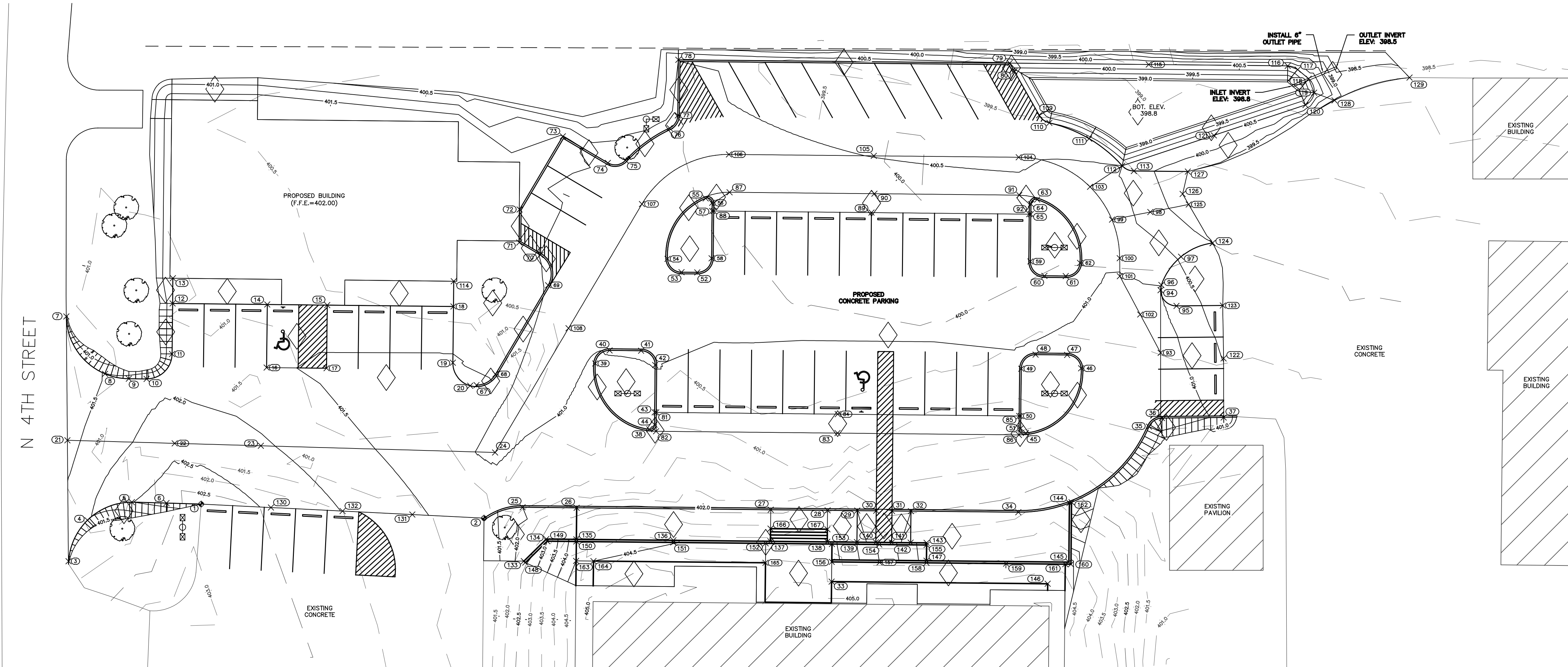
CALCULATIONS BASED OFF OF 25YR STORM USING RATIONAL METHOD.

POST DEVELOPED DRAINAGE CALCULATIONS (PHASE 2)



Issue/Revision	Date & Description
IFC: 5-25-22	

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SUBGRADE NOTES

SUBGRADE PREPARATION:  
1. SUBGRADE PREPARATION REQUIREMENTS ARE BASED ON RECOMMENDATIONS PROVIDED BY EARTHLOK. THESE RECOMMENDATIONS STATE THAT THE PARKING LOT AREA BE CHEMICALLY INJECTED TO A DEPTH OF 3 FEET OR TO IMPENETRABLE MATERIAL IN ORDER TO REDUCE THE SWELL TO AN AVERAGE OF 1% OF INJECTED DEPTH. INJECTIONS SHALL BE ON A MAXIMUM OF 4 FOOT CENTERS.

CUT & FILL QUANTITIES

DESCRIPTION	CUT (CY)	FILL (CY)
CUT & FILL VOLUMES	334.58	459.21
TOTAL NET BALANCE	124.63 (FILL)	

NOTE: VOLUMES ARE IN PLACE VALUES.

Point Table				
Point #	Elevation	Northing	Easting	Description
1	402.86	6942898.74	2798136.21	Benchmark #1 (Existing Concrete Corner)
2	401.19	6942894.28	2798225.83	Benchmark #2 (Existing Concrete Corner)
3	401.76	6942880.63	2798094.18	Top of Concrete
4	401.94	6942893.63	2798100.14	Top of Concrete
5	402.49	6942899.35	2798114.26	Top of Concrete
6	402.71	6942899.04	2798125.24	Top of Concrete
7	401.00	6942958.20	2798093.48	Top of Concrete
8	401.52	6942940.02	2798105.75	Top of Concrete
9	401.68	6942938.46	2798113.28	Top of Concrete
10	401.80	6942938.41	2798118.91	Top of Concrete
11	401.80	6942946.32	2798126.99	Top of Concrete
12	401.80	6942962.33	2798127.16	Top of Concrete
13	401.88	6942970.33	2798127.25	Top of Concrete
14	401.80	6942962.01	2798157.16	Top of Concrete
15	401.80	6942961.80	2798176.16	Top of Concrete
16	401.60	6942942.01	2798156.95	Top of Concrete
17	401.40	6942941.80	2798175.94	Top of Concrete
18	401.80	6942961.37	2798216.16	Top of Concrete
19	401.44	6942943.54	2798215.96	Top of Concrete
20	401.19	6942936.23	2798221.45	Top of Concrete
21	401.38	6942918.96	2798093.81	Top of Concrete
22	402.40	6942917.99	2798127.82	Top of Concrete
23	402.07	6942917.21	2798155.12	Top of Concrete
24	400.90	6942915.09	2798229.36	Top of Concrete
25	402.00	6942897.72	2798238.06	Back of Curb
26	402.00	6942897.55	2798255.19	Back of Curb
27	402.00	6942896.94	2798316.68	Back of Curb
28	402.00	6942896.76	2798334.70	Back of Curb

Point Table				
Point #	Elevation	Northing	Easting	Description
29	402.00	6942896.67	2798344.10	Back of Curb
30	401.50	6942896.61	2798350.10	Back of Curb
31	401.50	6942896.56	2798355.10	Back of Curb
32	402.00	6942896.50	2798361.10	Back of Curb
33	404.85	6942874.16	2798335.96	Back of Curb
34	402.00	6942896.16	2798395.14	Back of Curb
35	402.00	6942923.26	2798436.54	Back of Curb
36	402.00	6942926.01	2798440.64	Back of Curb
37	401.35	6942926.18	2798460.21	Back of Curb
38	401.75	6942922.44	2798278.19	Back of Curb
39	401.54	6942943.30	2798261.07	Back of Curb
40	401.48	6942947.45	2798265.61	Back of Curb
41	401.48	6942947.35	2798275.61	Back of Curb
42	401.48	6942942.81	2798280.06	Back of Curb
43	401.68	6942927.81	2798279.91	Back of Curb
44	401.72	6942923.92	2798279.87	Back of Curb
45	401.75	6942921.25	2798397.52	Back of Curb
46	401.60	6942941.76	2798415.05	Back of Curb
47	401.56	6942946.01	2798410.61	Back of Curb
48	401.53	6942946.11	2798400.60	Back of Curb
49	401.53	6942941.65	2798396.05	Back of Curb
50	401.68	6942926.65	2798395.91	Back of Curb
51	401.72	6942922.76	2798395.87	Back of Curb
52	401.33	6942972.18	2798293.41	Back of Curb
53	401.36	6942972.23	2798288.41	Back of Curb
54	401.32	6942976.47	2798283.96	Back of Curb
55	401.20	6942995.63	2798296.06	Back of Curb
56	401.14	6942994.23	2798298.13	Back of Curb

Point Table				
Point #	Elevation	Northing	Easting	Description
57	401.19	6942991.63	2798298.11	Back of Curb
58	401.30	6942976.63	2798297.96	Back of Curb
59	401.33	6942975.63	2798398.95	Back of Curb
60	401.37	6942971.08	2798403.41	Back of Curb
61	401.40	6942971.01	2798410.16	Back of Curb
62	401.40	6942975.17	2798414.69	Back of Curb
63	401.15	6942995.26	2798401.11	Back of Curb
64	401.19	6942993.85	2798399.13	Back of Curb
65	401.19	6942990.63	2798399.10	Back of Curb
67	401.19	6942936.46	2798223.46	Back of Curb
68	401.65	6942939.90	2798229.49	Back of Curb
69	401.48	6942968.11	2798246.16	Back of Curb
70	401.48	6942977.70	2798243.69	Back of Curb
71	401.68	6942981.65	2798237.00	Back of Curb
72	401.68	6942992.11	2798237.10	Back of Curb
73	401.53	6943015.31	2798250.81	Back of Curb
74	401.46	6943006.87	2798265.08	Back of Curb
75	401.43	6943007.70	2798270.68	Back of Curb
76	401.34	6943017.91	2798284.64	Back of Curb
77	401.31	6943021.95	2798287.24	Back of Curb
78	401.00	6943039.56	2798287.42	Back of Curb
79	400.55	6943038.52	2798392.40	Back of Curb
80	400.52	6943035.91	2798393.87	Back of Curb
81	401.18	6942927.80	2798280.41	Top of Concrete
82	401.25	6942921.80	2798280.35	Top of Concrete
83	401.25	6942921.23	2798337.85	Top of Concrete
84	401.18	6942927.23	2798337.91	Top of Concrete
85	401.18	6942926.66	2798395.41	Top of Concrete

Point Table				
Point #	Elevation	Northing	Easting	Description
86	401.25	6942920.66	2798395.35	Top of Concrete
87	400.70	6942997.58	2798303.63	Top of Concrete
88	400.69	6942991.63	2798298.61	Top of Concrete
89	400.69	6942991.13	2798348.60	Top of Concrete
90	400.65	6942997.12	2798349.54	Top of Concrete
91	400.65	6942996.66	2798395.45	Top of Concrete
92	400.69	6942990.63	2798398.60	Top of Concrete
93	401.28	6942946.68	2798440.22	Top of Concrete
94	401.07	6942966.54	2798440.20	Top of Concrete
95	400.79	6942961.55	2798445.25	Top of Concrete
96	400.99	6942968.17	2798440.47	Top of Concrete
97	400.43	6942977.13	2798446.69	Top of Concrete
98	400.27	6942991.31	2798436.97	Top of Concrete
99	400.83	6942988.91	2798424.82	Top of Concrete
100	400.83	6942976.66	2798427.25	Top of Concrete
101	401.05	6942970.97	2798427.30	Top of Concrete
102	401.17	6942958.82	2798433.76	Top of Concrete
103	400.75	6942999.29	2798417.88	Top of Concrete
104	400.40	6943008.66	2798395.25	Top of Concrete
105	400.49	6943009.12	2798349.34	Top of Concrete
106	400.59	6943009.58	2798303.43	Top of Concrete
107	400.67	6942993.85	2798275.88	Top of Concrete
108	400.79	6942954.47	2798252.62	Top of Concrete
109	400.00	6943021.99	2798401.73	Top of Concrete
110	400.00	6943019.57	2798404.98	Top of Concrete
111	400.23	6943014.57	2798417.58	Top of Concrete
112	400.50	6943005.98	2798428.06	Top of Concrete
113	400.30	6943004.31	2798431.74	Top of Concrete

Point Table				
Point #	Elevation	Northing	Easting	Description
114	401.88	6942969.37	2798216.24	Top of Concrete
115	400.50	6943038.08	2798436.45	Top of Berm
116	400.50	6943037.64	2798480.49	Top of Berm
117	400.50	6943036.47	2798483.66	Top of Berm
118	400.50	6943033.56	2798487.13	Top of Berm
119	400.50	6943029.12	2798488.88	Top of Berm
120	400.50	6943025.24	2798486.11	Top of Berm
121	400.50	6943015.61	2798457.09	Top of Berm
122	400.48	6942944.91	2798460.16	Top of Existing Concrete
123	400.26	6942961.68	2798459.92	Top of Existing Concrete
124	399.99	6942981.49	2798456.69	Top of Existing Concrete
125	399.71	6942993.70	2798449.12	Top of Existing Concrete
126	399.63	6942997.03	2798447.19	Top of Existing Concrete
127	399.51	6943004.14	2798448.85	Top of Existing Concrete
128	398.92	6943026.69	2798495.32	Top of Existing Concrete
129	398.72	6943033.92	2798519.10	Top of Existing Concrete
130	402.43	6942897.64	2798158.31	Top of Existing Concrete
131	401.59	6942895.41	2798203.12	Top of Existing Concrete
132	401.99	6942896.51	2798181.02	Top of Existing Concrete
133	402.25	6942880.65	2798238.44	Bottom of Wall
134	402.21	6942887.65	2798245.59	Bottom of Wall
135	402.25	6942887.56	2798255.09	Bottom of Wall
136	402.25	6942896.51	2798181.02	Bottom of Wall
137	402.25	6942886.95	2798316.58	Bottom of Wall
138	402.25	6942886.77	2798334.60	Bottom of Wall
139	402.25	6942886.67	2798344.00	Bottom of Wall
140	401.75	6942886.62	2798350.00	Bottom of Wall
141	401.75	6942886.57	2798355.00	Bottom of Wall

Point Table				
Point #	Elevation	Northing	Easting	Description
142	402.25	6942886.51	2798361.00	Bottom of Wall
143	402.25	6942886.46	2798366.09	Bottom of Wall
144	402.00	6942899.04	2798411.32	Bottom of Wall
145	402.30	6942880.34	2798411.13	Bottom of Wall
146	404.75	6942873.48	2798404.44	Bottom of Wall
147	402.30	6942880.79	2798366.03	Bottom of Wall
148	402.25	6942880.17	2798238.92	Top of Wall
149	403.00	6942886.97	2798245.86	Top of Wall
150	404.35	6942886.88	2798255.08	Top of Wall
151	404.49	6942886.58	2798285.80	Top of Wall
152	404.63	6942886.29	2798315.08	Top of Wall
153	404.63	6942886.07	2798336.08	Top of Wall
154	403.44	6942885.93	2798351.08	Top of Wall
155	402.25	6942885.78	2798366.08	Top of Wall
156	404.71	6942880.41	2798336.02	Top of Wall
157	404.71	6942880.26	2798351.18	Top of Wall
158	404.71	6942880.11	2798366.03	Top of Wall
159	404.70	6942879.86	2798391.27	Top of Wall
160	404.70	6942879.66	2798411.80	Top of Wall
161	404.70	6942879.67	2798410.01	Top of Wall
162	402.00	6942899.30	2798412.00	Top of Wall
163	404.50	6942880.72	2798255.02	Top of Concrete
164	404.50	6942880.66	2798260.52	Top of Concrete
165	404.70	6942880.12	2798315.02	Top of Concrete
166	402.13	6942890.94	2798316.62	Top of Concrete
167	402.13	6942890.76	2798334.64	Top of Concrete

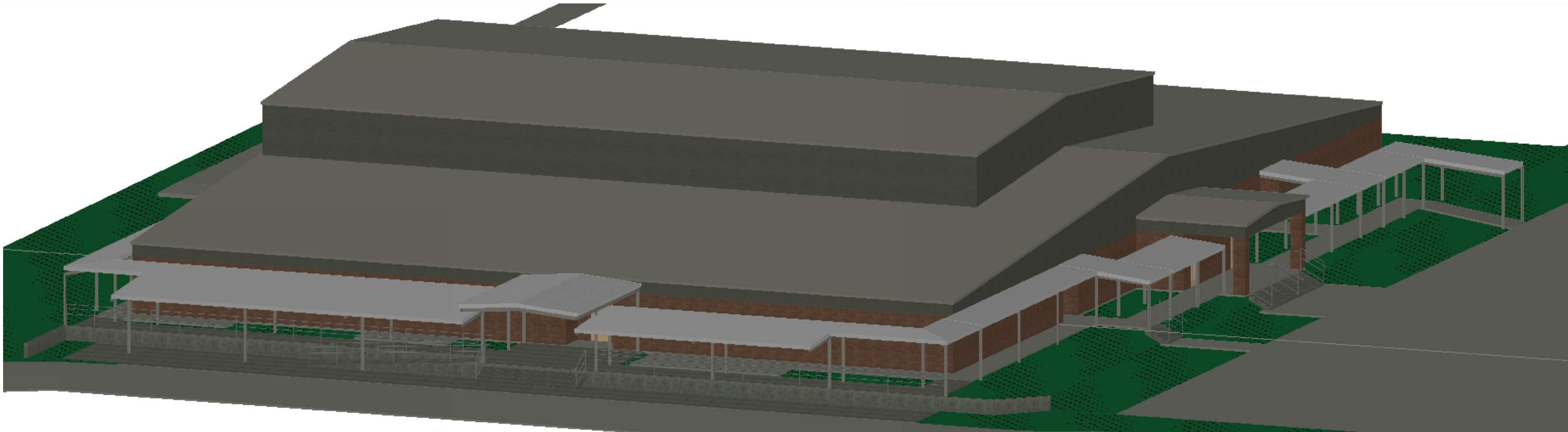
GENERAL NOTES

GENERAL NOTES

1. THESE DRAWINGS ARE ISSUED TO INDICATE THE GENERAL SCOPE OF THE PROJECT IN TERMS OF THE STRUCTURAL DESIGN CONCEPT.
2. CONTRACTOR IS TO FURNISH ALL ITEMS REQUIRED FOR PROPER COMPLETION OF THE WORK WITHOUT ADJUSTMENT TO CONTRACT PRICE. COMPLETED WORK SHALL BE OF SOUND AND QUALITY CONSTRUCTION AND THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE INCLUSION OF ADEQUATE AMOUNTS TO COVER INSTALLATION OF ALL ITEMS INDICATED, DESCRIBED OR IMPLIED, TO PERFORM THE INTENDED FUNCTIONS SPECIFIED IN THE CONSTRUCTION DOCUMENTS.
3. THE DETAILS DESIGNATED AS "TYPICAL DETAILS" APPLY GENERALLY TO THE DRAWINGS IN AREAS WHERE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED IN THE DETAILS, UNLESS NOTED OTHERWISE. TYPICAL SECONDARY FRAMING AND PANEL/TRIM DETAILS ARE SHOWN FOR DESIGN INTENT. SUPPLIER/INSTALLER SHALL PROVIDE THEIR STANDARD SECONDARY FRAMING AND PANEL/TRIM DETAILS TO ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
4. ALL DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS. DO NOT SCALE THE DRAWINGS. DIMENSIONS ARE TO FACE OF COLUMN (F.O.C.), CENTER OF COLUMN (C.O.C.) FACE OF CHANNEL (F.O.CH.), FACE OF WALL (F.O.W.), FACE OF EXISTING STRUCTURE AND EXTERIOR FACE OF ANGLE LEGS.
5. PRINCIPAL OPENINGS, CURBS AND SLAB DEPRESSIONS ARE SHOWN ON THE DRAWINGS. SEE ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, OTHER OPENINGS AND SLAB DEPRESSIONS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, CURBS, AND SLAB DEPRESSIONS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. SIZES AND LOCATIONS OF OPENINGS, CURBS AND DEPRESSIONS SHALL BE VERIFIED BY THE CONTRACTOR BY REVIEWING LAYOUT OF THE AREA ON THE ARCHITECTURAL DRAWINGS.
6. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DRAWINGS TO CONFIRM ALL REQUIREMENTS OF THE WORK. REPORT ANY CONFLICT/DISCREPANCY BETWEEN THE DISCIPLINES TO THE DESIGN PROFESSIONAL PRIOR TO EXECUTION OF THE WORK.
7. THE CONTRACTOR SHALL INSURE THAT ALL CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS AND THAT THESE LOADS ARE NOT PUT ON THE STRUCTURAL MEMBERS PRIOR TO THE TIME THAT THE FRAMING MEMBERS, BRACING MEMBERS AND CONNECTIONS ARE IN PLACE.
8. THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING AND SHORING REQUIRED FOR COMPLETION OF THE WORK.
9. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS EMPLOYED FOR THE COMPLETION OF THE PROJECT.
10. THE CONTRACTOR SHALL OBTAIN ALL PERMITS NECESSARY FOR COMPLETION OF THE WORK.
11. THE CONTRACTOR SHALL ARRANGE ALL REQUIRED INSPECTIONS.

DESIGN CODES AND LOADS

1. BUILDING CODE: 2015 IBC
2. STRUCTURAL CONCRETE: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318.
3. STRUCTURAL STEEL: MANUAL OF STEEL CONSTRUCTION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
4. MASONRY: BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTURES, ACI 530.
5. DESIGN WIND LOAD CRITERIA:
  - RISK CATEGORY 2
  - WIND VELOCITY – 115MPH
  - EXPOSURE – C
  - OPEN BUILDING
  - DESIGNED WITH DIRECTION PROCEDURE.
6. DESIGN LIVE LOAD:
  - ROOF 5PSF (CANOPY)
7. DESIGN DEAD LOAD:
  - BUILDING COMPONENT SELF WEIGHT
  - COLLATERAL 5PSF



CONCRETE NOTES

CAST-IN-PLACE CONCRETE:

1. MATERIALS
  - A. PORTLAND CEMENT SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR PORTLAND CEMENT, ASTM DESIGNATION C-150, LATEST VERSION TYPE II.
  - B. CONCRETE AGGREGATES, ASTM C-33. THE MAXIMUM SIZE SHALL NOT BE LARGER THAN 1½" INCHES FOR SLABS ON GRADE, ¾" INCHES FOR CONCRETE FILL OVER FORMED OR COMPOSITE DECKS.
  - C. WATER SHALL BE CLEAN, POTABLE, AND FREE OF INJURIOUS AMOUNTS OF ACIDS, ALKALOIDS, OR ORGANIC MATTER.
  - D. READY MIX CONCRETE SHALL CONFORM TO ASTM SPECIFICATION C-94, LATEST VERSION.
  - E. ADMIXTURES USED TO ASSIST THE WORKABILITY OF CONCRETE SHALL CONFORM TO ASTM SPECIFICATIONS C-260 AND C-494, LATEST VERSION. ADMIXTURES MUST BE APPROVED BY THE ENGINEER.
2. CAST-IN-PLACE CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH AT 28 DAYS OF 4000 PSI, A MIN. SLUMP 3", MAX SLUMP 5", MIN. AIR CONTENT 3%, MAX. AIR CONTENT 5%, MIN. WATER TO CEMENT RATIO .45, MAX WATER TO CEMENT RATIO .55.
3. INTERIOR CONCRETE SLAB SURFACE SHALL RECEIVE FLOAT FINISH. EXTERIOR SLAB SURFACES SHALL RECEIVE MEDIUM BROOM FINISH. SEAL SLAB WITH CLEAR CONCRETE SEALER EQUIVALENT TO PRODUCTS PRODUCED BY WR MEADOWS OR SIKA.
4. CONTINUOUS REINFORCING BARS SHALL HAVE A MINIMUM OVERLAP OF 30 DIAMETERS OR 24 INCHES, WHICHEVER IS GREATER.
5. PROVIDE CORNER BARS IN GRADE BEAMS AND FOOTINGS AT EACH CORNER AND/OR INTERSECTION (TOP AND BOTTOM).
6. PROVIDE CONTROL JOINTS (SAWED, FORMED OR TOOLED) WHERE SHOWN ON PLAN, PROVIDE CONSTRUCTION JOINTS WHERE SECTIONS CAN NOT BE PLACED CONTINUOUSLY.
  - 6.1. FOR MAT SLAB JOINTS PROVIDE SIKA LOADFLEX –524 EZ (OR EQUAL) TWO-COMPONENT, QUICK-SETTING, SEMI-RIGID, SOLVENT-FREE, SELF-LEVELING CONTROL JOINT FILLER AFTER CONTROL JOINTS ARE PLACED. SELECTED FILLER SHALL HAVE A SHORE D HARDNESS OF 30-32 AND SHORE A HARDNESS OF 80-85.
  - 6.2. FOR EXTERIOR SLAB/WALL JOINTS PROVIDE MIN.¼" DEEP JOINT SEALANT OVER BACKER ROD. PROVIDE W.R. MEADOWS POURTHANE NS NON-SAG JOINT SEALANT OR EQUIVALENT.
7. PROVIDE ½" WIDE MINIMUM EXPANSION/ISOLATION JOINTS IN FLOOR SLAB AS SHOWN ON DRAWINGS. EXPANSION/ISOLATION JOINTS, SHALL HAVE PRE-MOLDED JOINT FILLERS THAT MEET THE REQUIREMENTS OF ASTM SPECIFICATIONS D-994, LATEST VERSION. JOINT SHALL HAVE ½" DEEP SELF-LEVELING JOINT SEALANT. PROVIDE W.R. MEADOWS FOURTHANE NS NON-SAG JOINT SEALANT OR EQUIVALENT.
8. ALL REINFORCING SHALL BE SUPPORTED ON METAL OR PLASTIC SUPPORTS AND SECURELY TIED TO PREVENT MOVEMENT DURING CONCRETING.
9. REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60, EXCEPT TIES AND STIRRUPS MAY BE GRADE 40.
10. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, FABRIC SHALL BE SUPPLIED IN FLAT SHEETS AND SHALL BE LAPPED 2 MESHS AT SPLICES.
11. CONCRETE PROTECTION FOR REINFORCING BARS: DEPOSITED AGAINST EARTH 3", EXPOSED TO WEATHER OR EARTH 2", 1½" AT TOP OF INTERIOR SLABS.
12. CONDUIT FOR ELECTRICAL OR PLUMBING LINES SHALL BE SLEEVED THROUGH GRADE BEAMS.
13. ALL DRILLED/EPOXY SET ANCHORS OR DOWELS SHALL BE AS MANUFACTURED BY HILTI. CAST IN ANCHORS SHALL BE ASTM F1554 GRADE 36.
14. CONTRACTOR SHALL MAKE PROVISIONS FOR FINISHING EXPOSED CONCRETE. AS SOON AS FORMS ARE REMOVED, UNDESIRABLE FINS AND PROJECTIONS SHALL BE REMOVED, OFFSETS SHALL BE LEVELED, AND VOID OR DAMAGED PLACES SHALL BE IMMEDIATELY SATURATED WITH WATER AND FINISHED WITH CEMENTIOUS PASTE OR MORTAR OF THE SAME COMPOSITION AS USED IN THE MIX. EXPOSED SURFACES SHALL BE RUBBED WITH CARBORUNDUM STONE TO A SMOOTH FINISH FREE FROM FORM MARKS OR HONEYCOMBS.
15. CONCRETE SHALL BE PLACED CONTINUOUSLY, WITHOUT INTERRUPTION, IN SUCH A MANNER AS NOT TO CAUSE SEGREGATION. EACH LIFT SHALL BE VIBRATED. WHERE CONCRETE DROPS ARE MORE THAN 5 FEET, CONCRETE SHALL BE PLACED USING A CHUTE, TREMIE, OR PUMP HOSE.
16. FOUNDATION CONTRACTOR SHALL PROVIDE AND INSTALL ALL ANCHOR BOLTS AND EMBEDDED ITEMS.
17. TESTING LABORATORY SERVICES FOR QUALITY CONTROL OF CAST-IN-PLACE CONCRETE SHALL BE PROVIDED BY THE OWNER. TESTING SERVICES SHALL INCLUDE PREPARATION AND TESTING OF CONCRETE COMPRESSION CYLINDERS AND SLUMP TESTS AS DIRECTED BY THE OWNER'S REPRESENTATIVE. REPORTS SHALL BE SUBMITTED IN DUPLICATE TO THE ENGINEER.
  - 17.1. TEST CYLINDERS: DURING PROGRESS OF WORK, MOLD, CURE AND TEST SPECIMENS OF EACH DIFFERENT MIX DESIGN PLACED IN ANY ONE DAY. FOR EACH 50 CUBIC YARDS OF CONCRETE PLACED, OR PART THEREOF OVER 10 CUBIC YARDS, MAKE COMPRESSION TEST CYLINDERS DURING POUR. MOLD AND CURE TEST CYLINDERS IN ACCORDANCE WITH ASTM C-31. TEST CYLINDERS IN ACCORDANCE WITH ASTM C-39: ONE AT 7 DAYS AND 2 AT 28 DAYS. MAKE ADDITIONAL SETS OF 3 CYLINDERS WHEN OBVIOUS CHANGES IN MIX ARE APPARENT.
  - 17.2. SLUMP TESTS: MAKE SLUMP TESTS FOR EACH 50 CUBIC YARDS AT CONCRETE PLACED, AND FOR EACH SET OF CYLINDERS IN ACCORDANCE WITH ASTM C143.
  - 17.3. AIR CONTENT OF NORMAL-WEIGHT CONCRETE: DETERMINE TOTAL AIR CONTENT OF AIR ENTRAINED CONCRETE FOR EACH COMPRESSIVE STRENGTH TEST IN ACCORDANCE WITH ASTM C231.
18. FURNISH AND INSTALL 10 MIL POLYETHYLENE MEMBRANE VAPOR RETARDED UNDER ALL SLABS ON GRADE. VAPOR RETARDER SHEETS SHALL BE INSTALLED OVER LEVEL, COMPACTED GRANULAR SOIL WITH 6" JOINT LAPS. JOINT LAPS SHALL BE CONTINUOUSLY SEALED WITH DUCT TAPE. SEAL CAREFULLY AROUND ALL PIPES, CONDUITS, ETC., WHICH PENETRATE THE SLAB. SEAL ALL TEARS AND PUNCTURES PRIOR TO PLACING CONCRETE.

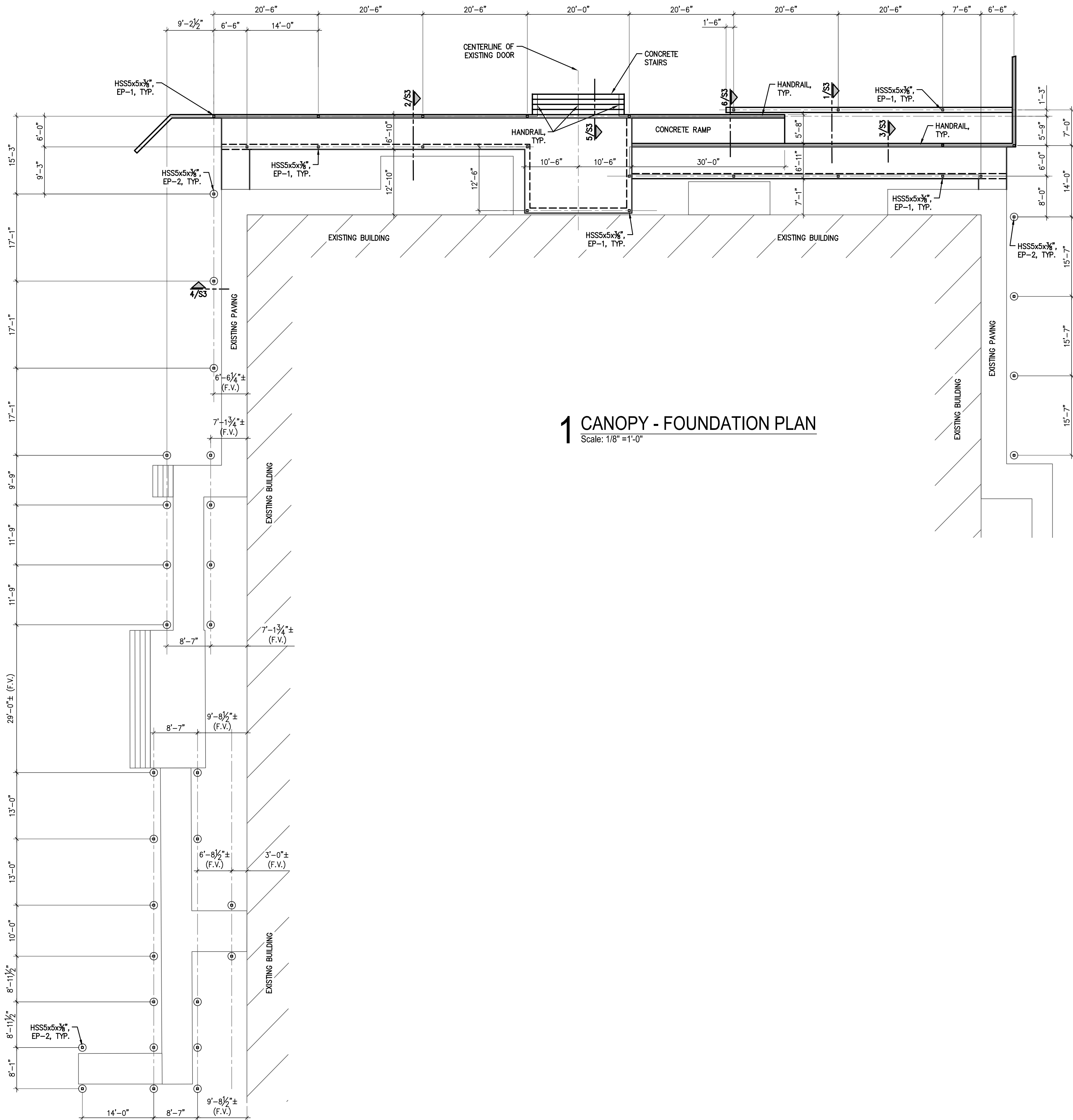
STEEL NOTES

1. STRUCTURAL BOLTS USED TO CONNECT HOT-ROLLED MEMBERS, BUILT UP MEMBERS, AND GIRT/PURLIN TO PRIMARY FRAMING SHALL BE ASTM A325 HIGH STRENGTH BOLTS, TYPE N. CONNECTIONS SHOWN THROUGH OUT PLANS SHALL BE ¾" MINIMUM UNLESS NOTED OTHERWISE. NUTS SHALL BE ASTM A563C AND HARDENED WASHERS SHALL BE ASTM F436.
2. ASTM A307 BOLTS SHALL BE PERMITTED FOR LADDER, STAIR TREAD, PURLIN TO GIRT, DOOR FRAME, AND HANDRAIL CONNECTIONS.
3. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36 AND GALVANIZED.
4. W MEMBERS SHALL BE ASTM A992 GRADE 50.
5. HSS MEMBERS SHALL BE ASTM A500 GRADE B.
6. THREADED RODS SHALL BE ASTM A36.
7. PIPE SHALL BE ASTM A53/A53M TYPES E OR S, GRADE B
8. PLATE USED FOR END PLATE MOMENT CONNECTIONS SHALL BE ASTM A529 GRADE 50.
9. HOT ROLLED CHANNELS, ANGLES, BARS, PLATES SHALL BE ASTM A36.
10. COLD-FORMED STRUCTURAL SHAPES SHALL CONFORM TO ANY OF THE FOLLOWING MATERIAL SPECIFICATIONS:
  - 10.1. ASTM A1011/A1011M, GRADE 55
  - 10.2. ASTM A1008/A1008M CLASS 1 OR 2, GRADE 55
  - 10.3. ASTM A653 SS CLASS 1, GRADE 50
11. ROOFING AND SIDING STEEL SHEETS SHALL CONFORM TO ANY OF THE FOLLOWING MATERIAL SPECIFICATIONS:
  - 11.0.1. ASTM A1011/A1011M – GRADE 50 OR 55
  - 11.0.2. ASTM A1008/A1008M – CLASS 1 OR 2, GRADE 50, OR HIGHER
  - 11.0.3. ASTM A653 – SS, EITHER GRADE 50, CLASS 1 OR 3, OR GRADE 80
  - 11.0.4. ASTM A792/A792M – SS, EITHER GRADE 50 TYPE A OR GRADE 80
12. ROOFING AND SIDING STEEL COATING SHALL CONFORM TO ANY OF THE FOLLOWING:
  - 12.0.1. ZINC COATING – ASTM A653 COATING DESIGNATION G-90
  - 12.0.2. ALUMINUM-ZINC ALLOY COATING – ASTM A792/A792M COATING DESIGNATION AZ-55
  - 12.0.3. PRIME/PAINTED – IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR SUPPLIER'S STANDARDS
13. UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, FLASHING, CORNER TRIM, CAPS, AND CLOSURE PIECES SHALL BE OF THE SAME MATERIAL, FINISH, AND COLOR AS ADJACENT MATERIAL.
14. TRIM PROFILE SHALL BE PROVIDED TO SUIT.
15. FASTENERS: WALL AND ROOFING THROUGH-FASTENERS SHALL BE SUPPLIER'S STANDARD SELF-DRILLING COATED STEEL SCREWS WITH SEALING WASHER. EXPOSED FASTENERS FOR EAVE, END LAP, RIDGE COVER, TRIM, AND FLASHING SHALL BE SUPPLIER'S STANDARD SELF-DRILLING STAINLESS STEEL SCREWS WITH SEALING WASHER. ALL EXPOSED FASTENER HEADS SHALL BE FACTORY COLORED TO MATCH COLOR OF PANELS.
16. SEALANT: FACTORY-APPLIED ROOF PANEL SEALANT SHALL BE NON-SHRINKING, NONDRYING, BUTYL-BASED SEALANT SPECIFICALLY FORMULATED FOR FACTORY APPLICATION IN STANDING SEAMS AND TO ALLOW ROOF PANEL ASSEMBLY AT TEMPERATURES FROM -23°C TO +60°C (-10°F TO +140°F) OR SUPPLIER'S STANDARD TYPE IF APPROVED BY THE PURCHASER. FIELD-APPLIED ROOF PANEL SEALANT SHALL BE A PURCHASER APPROVED, NON-SHRINKING, NON-DRYING, BUTYL-BASED SEALANT SPECIFICALLY FORMULATED FOR ROOF APPLICATION AT TEMPERATURES FROM 7°C TO 49°C (20°F TO 120°F) OR SUPPLIER'S STANDARD TYPE IF APPROVED BY THE PURCHASER.
17. PAINTING: UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, ALL HOT ROLLED AND BUILT-UP MEMBERS SHALL BE FACTORY-PRIMED. SURFACES SHALL BE CLEANED OF LOOSE MILL SCALE, RUST, DIRT, OIL, GREASE, AND OTHER MATTER THAN PRECLUDES THE BONDING OF PAINT, BEFORE BEING PRIMED. PRIMER SHALL BE APPLIED IN ACCORDANCE WITH THE PROCEDURES IN SSPC SP3, SSPC SP7, AND SSPC SP1, AS APPLICABLE. UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, MEMBERS SHALL BE PRIMED WITH THE SUPPLIER'S STANDARD RUST-INHIBITING PRIMER. UNLESS REQUIRED OTHERWISE IN THE CONTRACT DOCUMENTS, STEEL ROOFING, SIDING AND ALL RELATED TRIM SHALL BE COIL-COATED IN ACCORDANCE WITH ASTM A755/A755M. UNLESS OTHERWISE SPECIFIED IN THE CONTRACT DOCUMENTS, ROOF, SIDING, TRIM, GUTTERS, AND DOWNSPOUTS SHALL MATCH IN COLOR.
18. GROUT MATERIALS FOR LEVELING BASE PLATES SHALL BE PROPORTIONED PRODUCTS, IN ACCORDANCE WITH ASTM C1107/C1107M. COMPRESSIVE STRENGTH SHALL BE IN ACCORDANCE WITH ASTM C1107/C1107M (MINIMUM 28-DAY COMPRESSIVE STRENGTH, 5000 PSI).
19. PROVIDE STEEL BEAM CONNECTIONS AS DETAILED ON THE DRAWINGS.
20. STEEL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.
21. ANCHOR BOLTS HAVE NOT BEEN DESIGNED FOR ANY SPECIFIC ERECTION FORCES. THE ERECTOR IS RESPONSIBLE FOR ANY AND ALL CUTTING AND BRACING REQUIRED TO ERECT THE STRUCTURE.
22. THE CONTRACTOR SHALL MAKE SAFE PROVISIONS FOR STABILIZING THE STEEL STRUCTURE BOTH HORIZONTALLY AND VERTICALLY. STABILITY DURING ERECTION IS THE CONTRACTOR'S RESPONSIBILITY.

WELDING

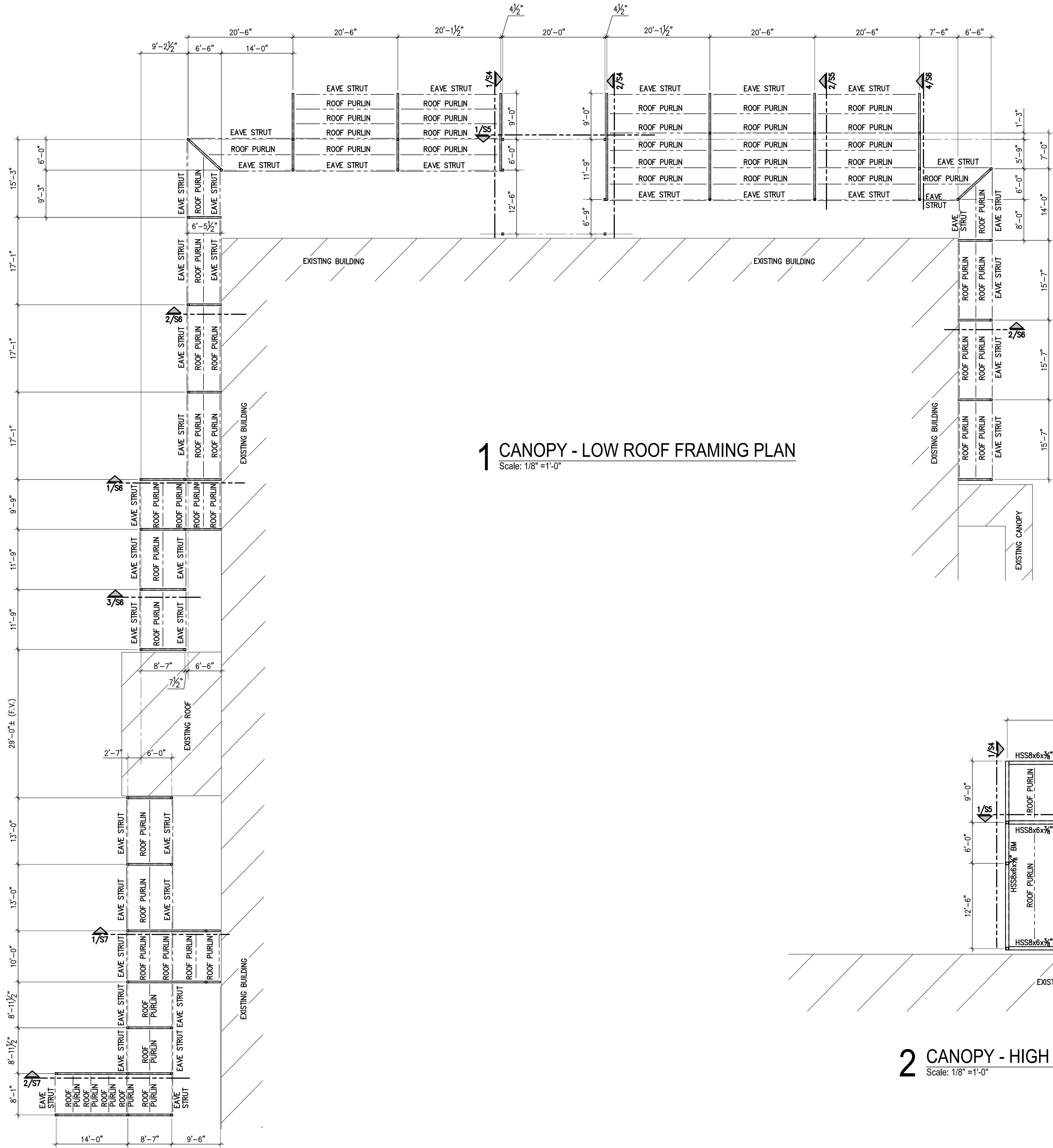
1. WELDED CONSTRUCTION SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE", LATEST EDITION.
2. ELECTRODES FOR FIELD AND SHOP WELDS OF STRUCTURAL STEEL SHALL BE E70XX, U.N.O.
3. WHEN WELDS ARE NOT CALLED-OUT ON DRAWINGS, THEY ARE MINIMUM SIZE CONTINUOUS FILLET WELDS IN ACCORDANCE WITH AWS. FILLET WELDS NOT SPECIFIED AS TO LENGTH SHALL BE CONTINUOUS.
4. UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL GROOVE WELDS SHALL BE FULL PENETRATION.
5. ONLY LOW HYDROGEN ELECTRODES SHALL BE USED ON REINFORCING STEEL AND ASTM A572 STEEL.

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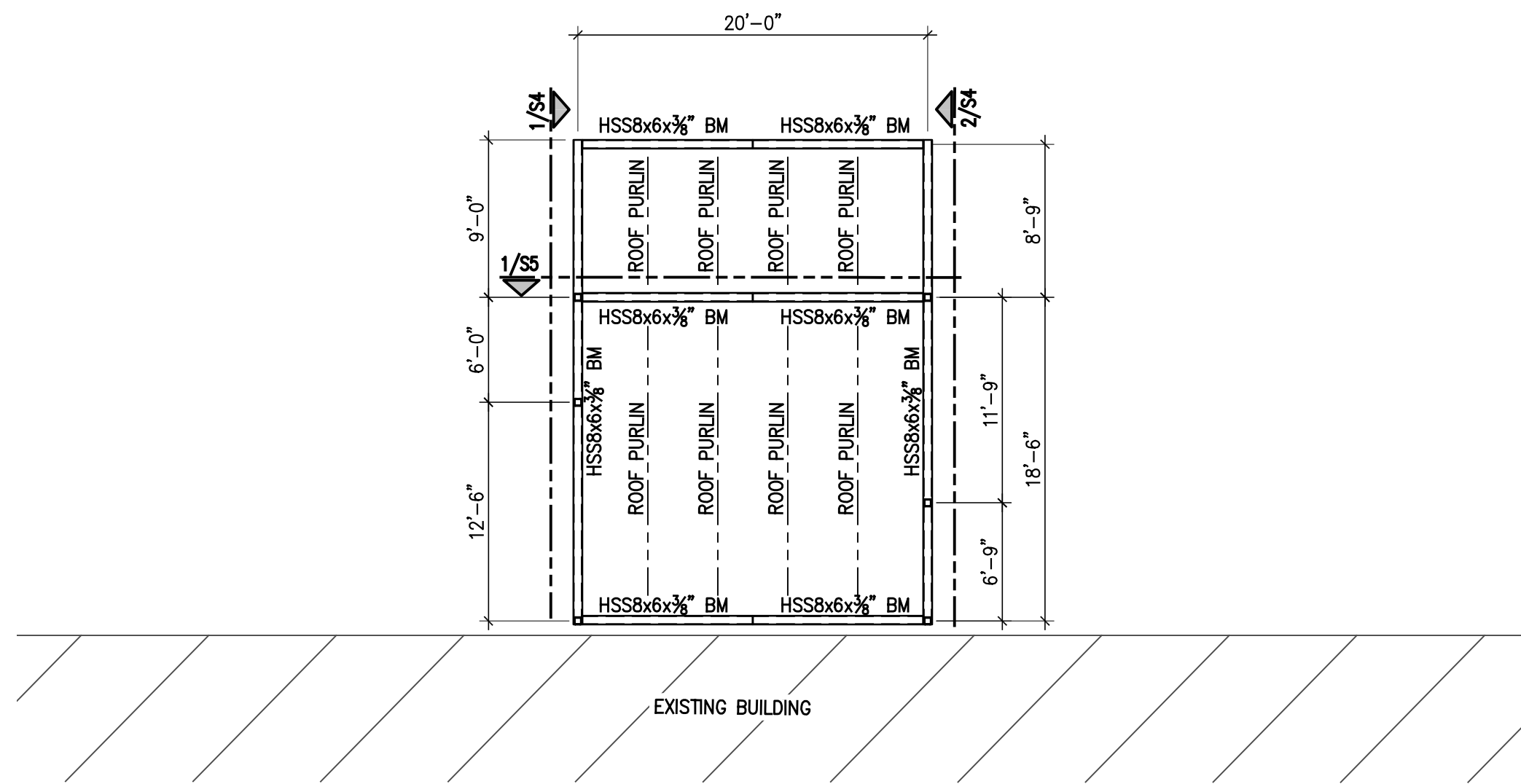


- PLAN NOTES:
1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO INSTALLATION OF CANOPIES.
  2. TOP OF ALL NEW CONCRETE SONOTUBE TYPE FOUNDATION ELEVATIONS SHALL MATCH THE TOP OF EXISTING CONCRETE SIDEWALK ELEVATIONS.
  3. SEE CIVIL GRADING PLAN FOR TOP OF CONCRETE SPOT ELEVATIONS.

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1 CANOPY - LOW ROOF FRAMING PLAN  
Scale: 1/8" = 1'-0"



2 CANOPY - HIGH ROOF FRAMING PLAN  
Scale: 1/8" = 1'-0"

- PLAN NOTES:
1. STEEL CONNECTIONS SHALL BE FIELD WELDED 1/4" FILLET WELDS (ALL AROUND).
  2. STEEL HSS COLUMNS AND BEAMS SHALL BE SHIPPED LONG AND FIELD CUT TO LENGTH.
  3. STEEL SHALL BE FACTORY PRIMED. COORDINATE WITH OWNER FOR COLOR OF PAINT, TRIM AND FINISH.
  4. SEE SHEET S7 FOR TYPICAL TRIM DETAILS.

Issue/Revision Date & Description

IFC: 5-25-22

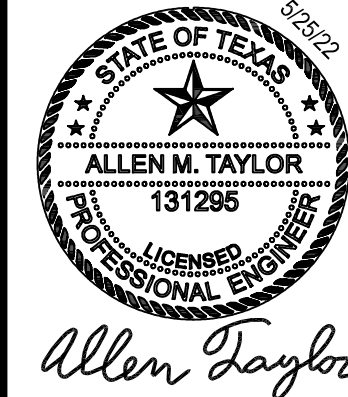
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050-21003

Sheet Number:

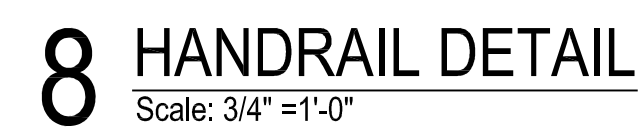
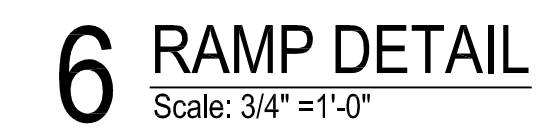
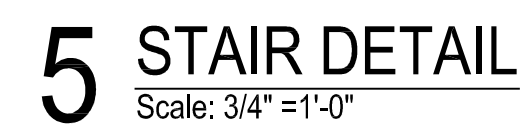
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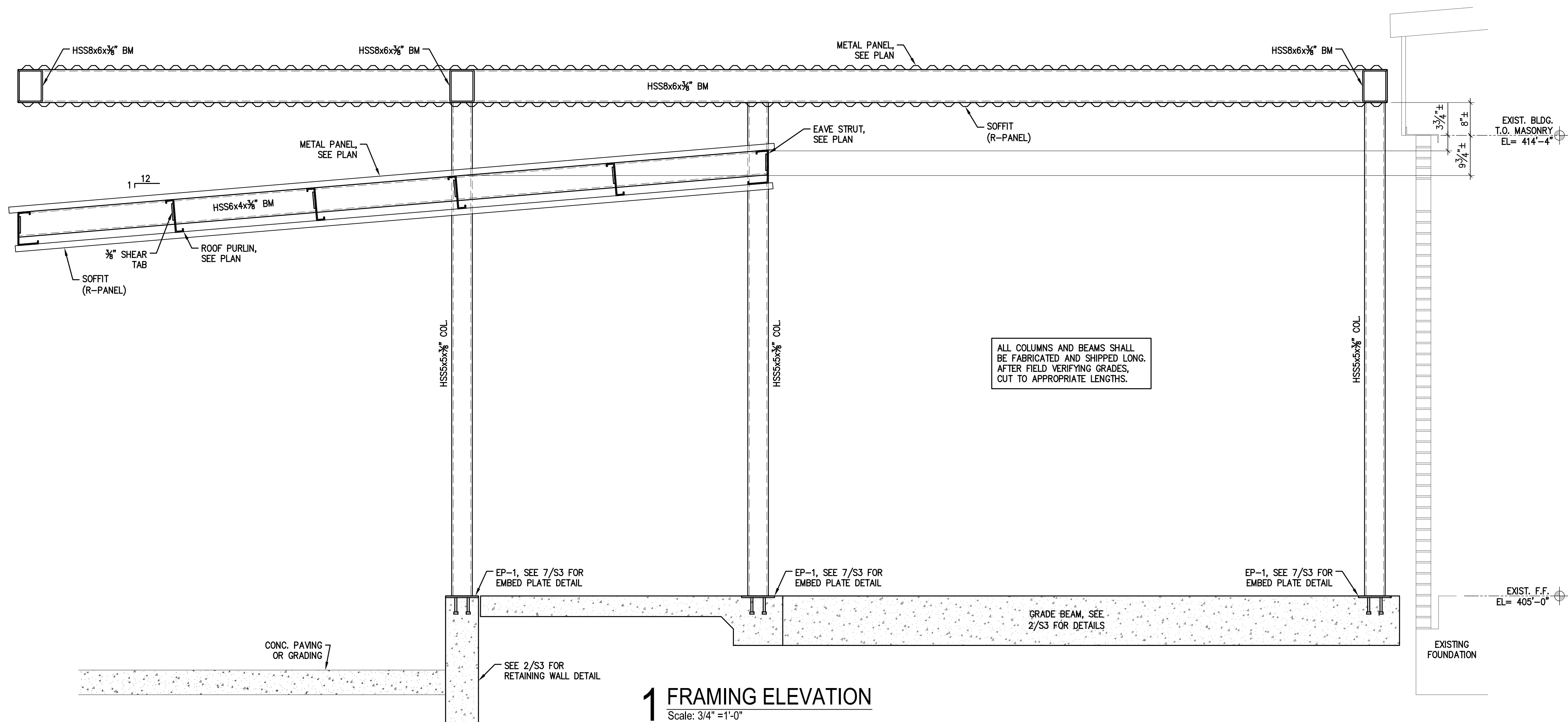
2021 FRUITVALE ISD  
ADMINISTRATION BLDG  
FRUITVALE, TEXAS 75127  
CANOPY - ROOF FRAMING PLAN



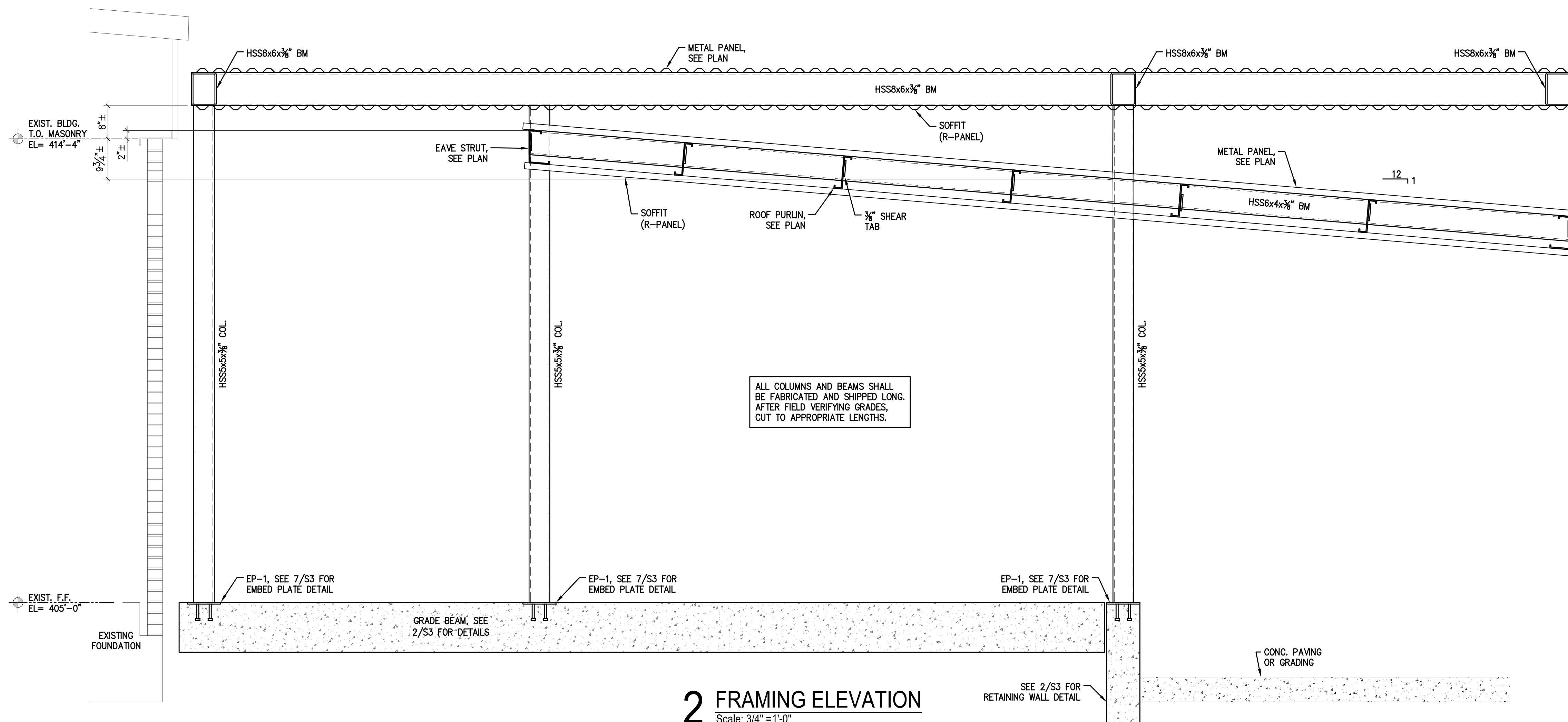
EXCEED  
ENGINEERING  
P.O. BOX 983  
CANTON, TX 75103  
903.473.8897  
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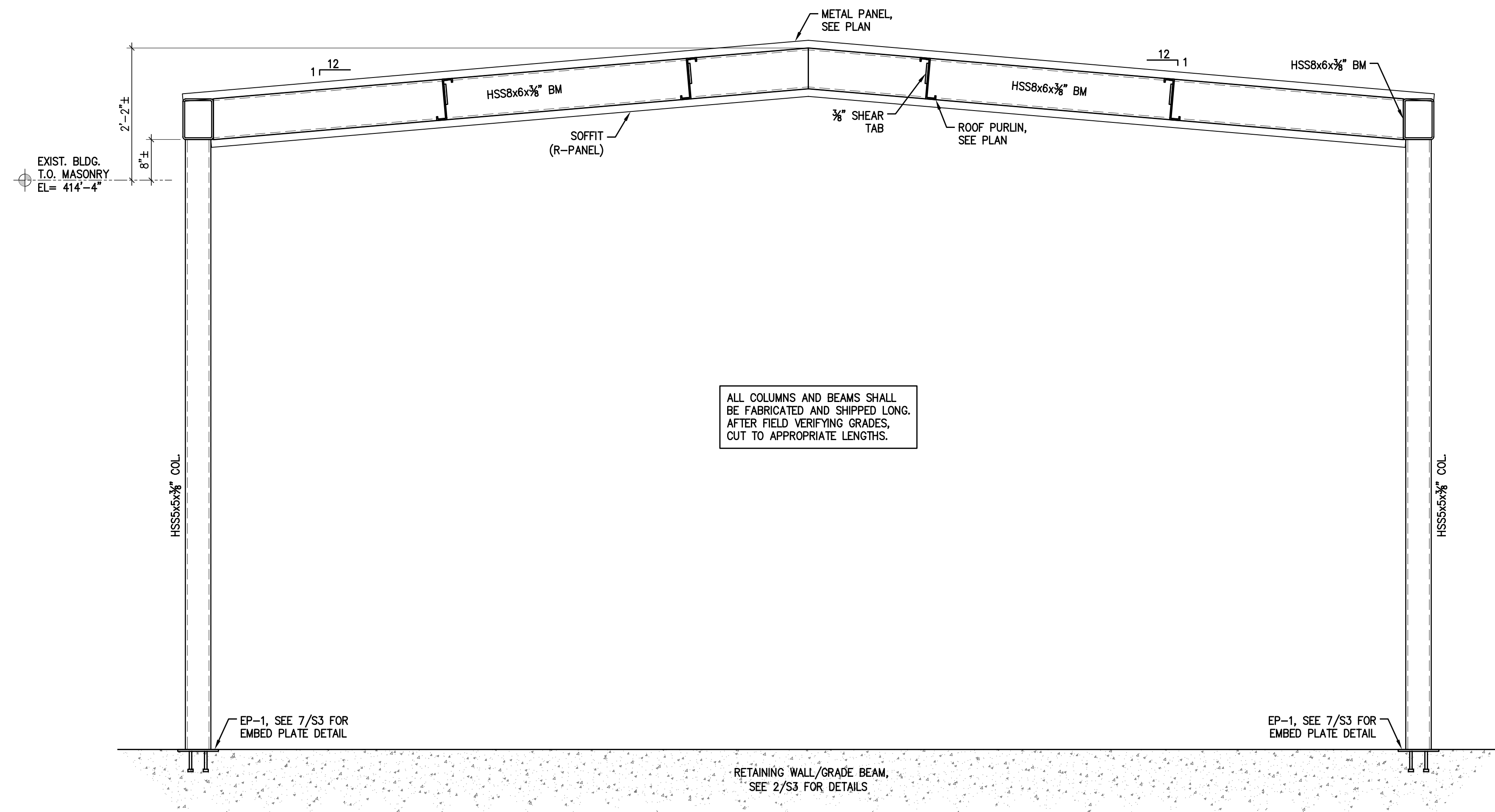


1 FRAMING ELEVATION  
Scale: 3/4" = 1'-0"

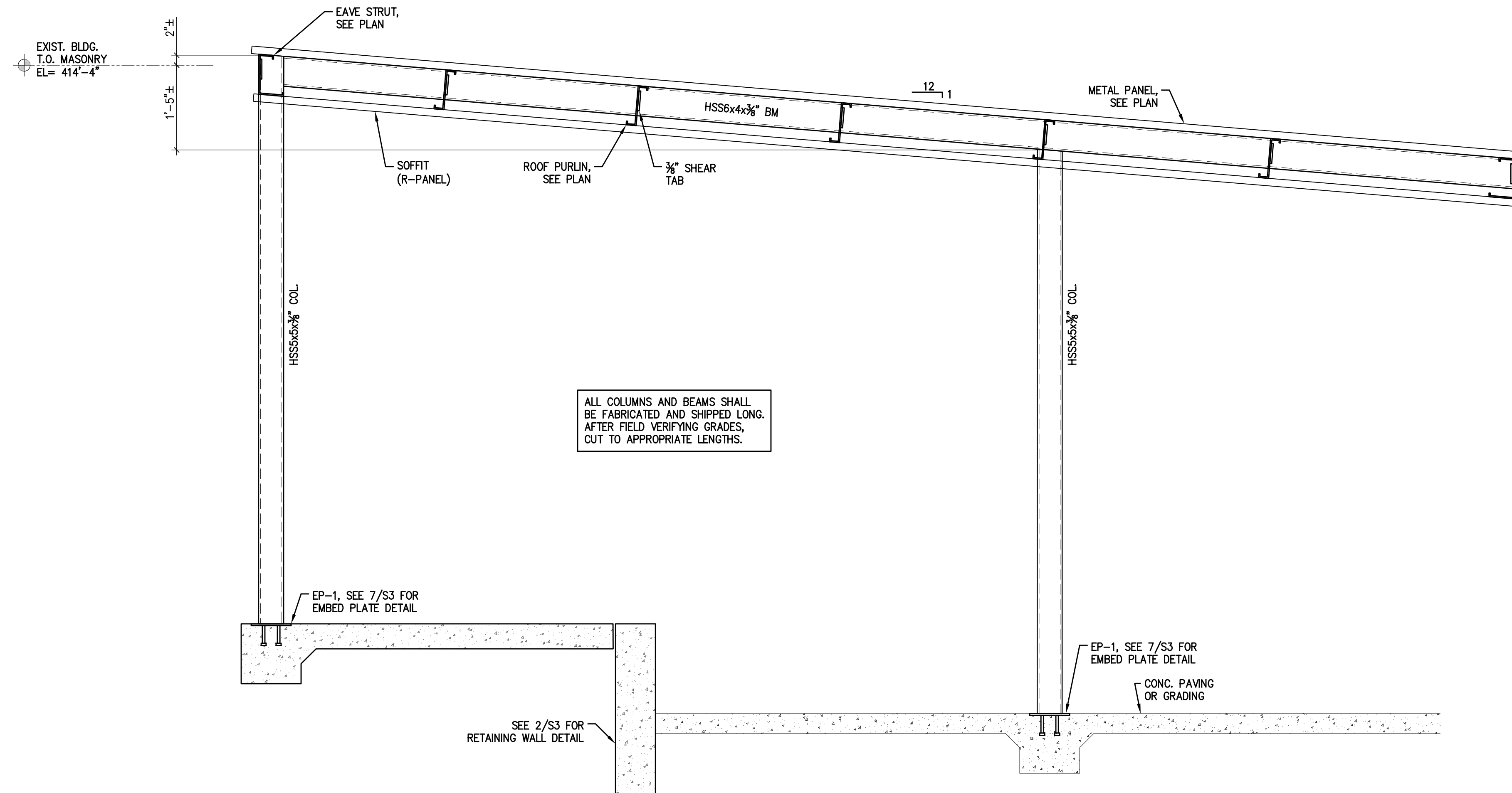


2 FRAMING ELEVATION  
Scale: 3/4" = 1'-0"

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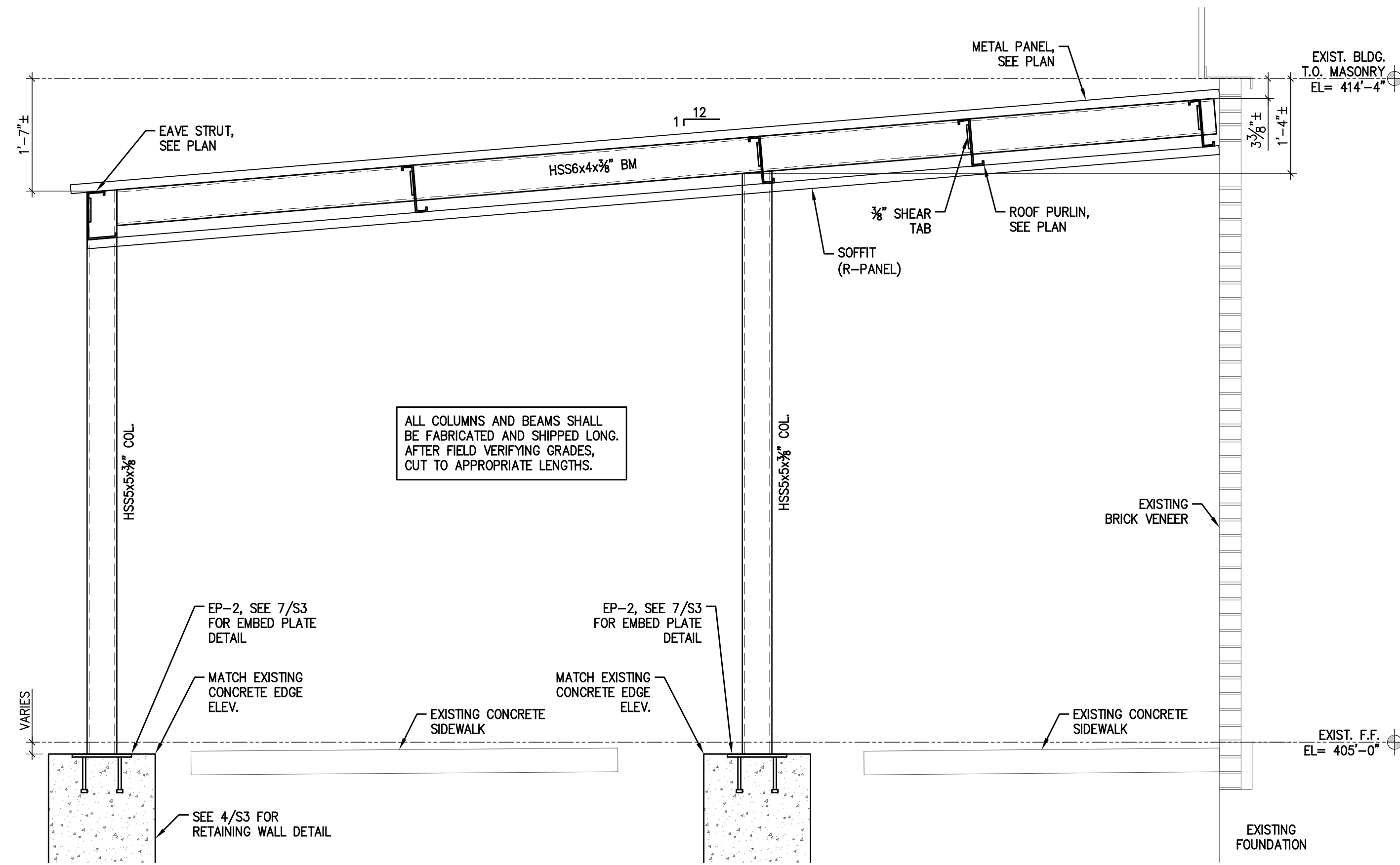


**1 FRAMING ELEVATION**  
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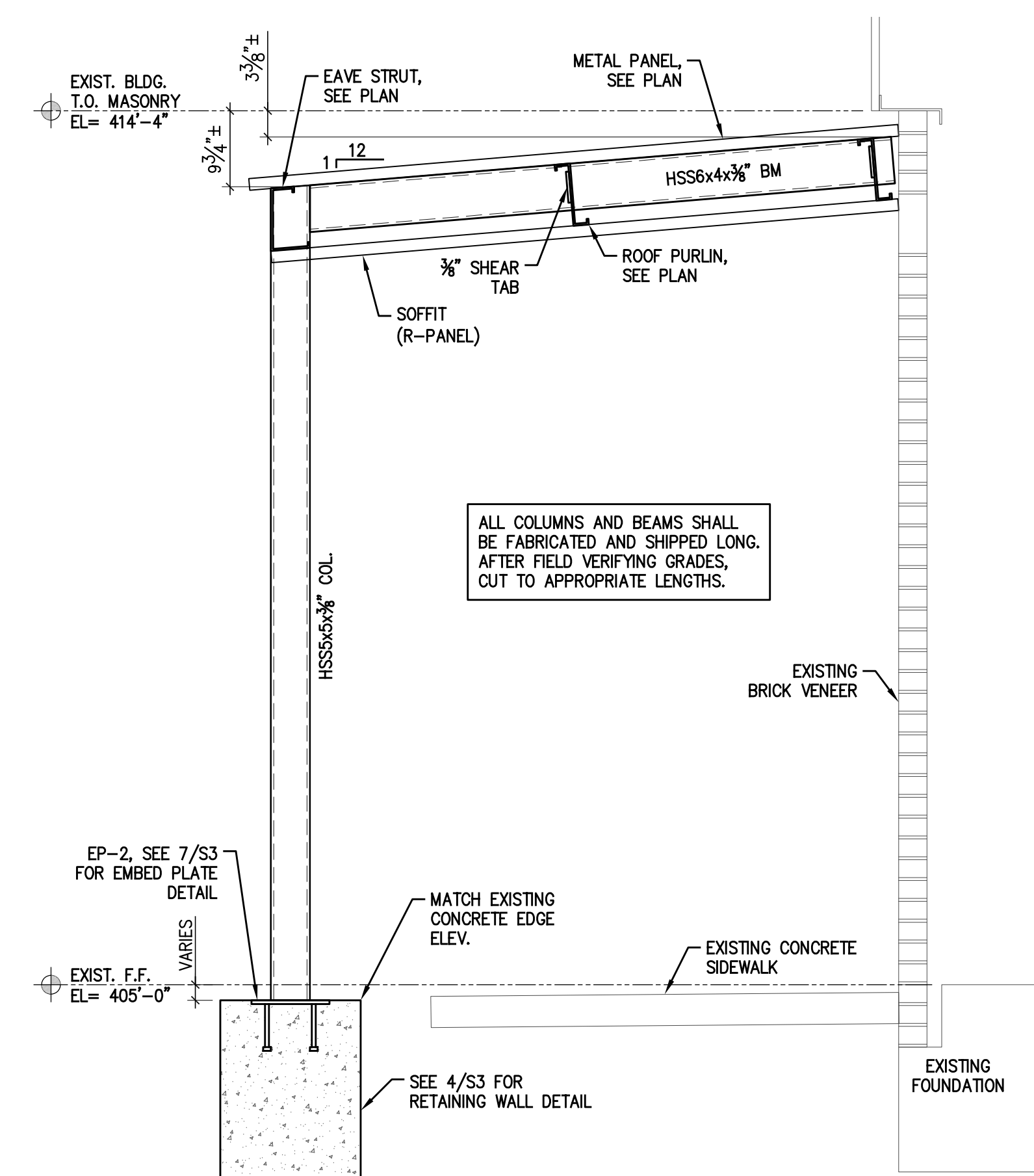


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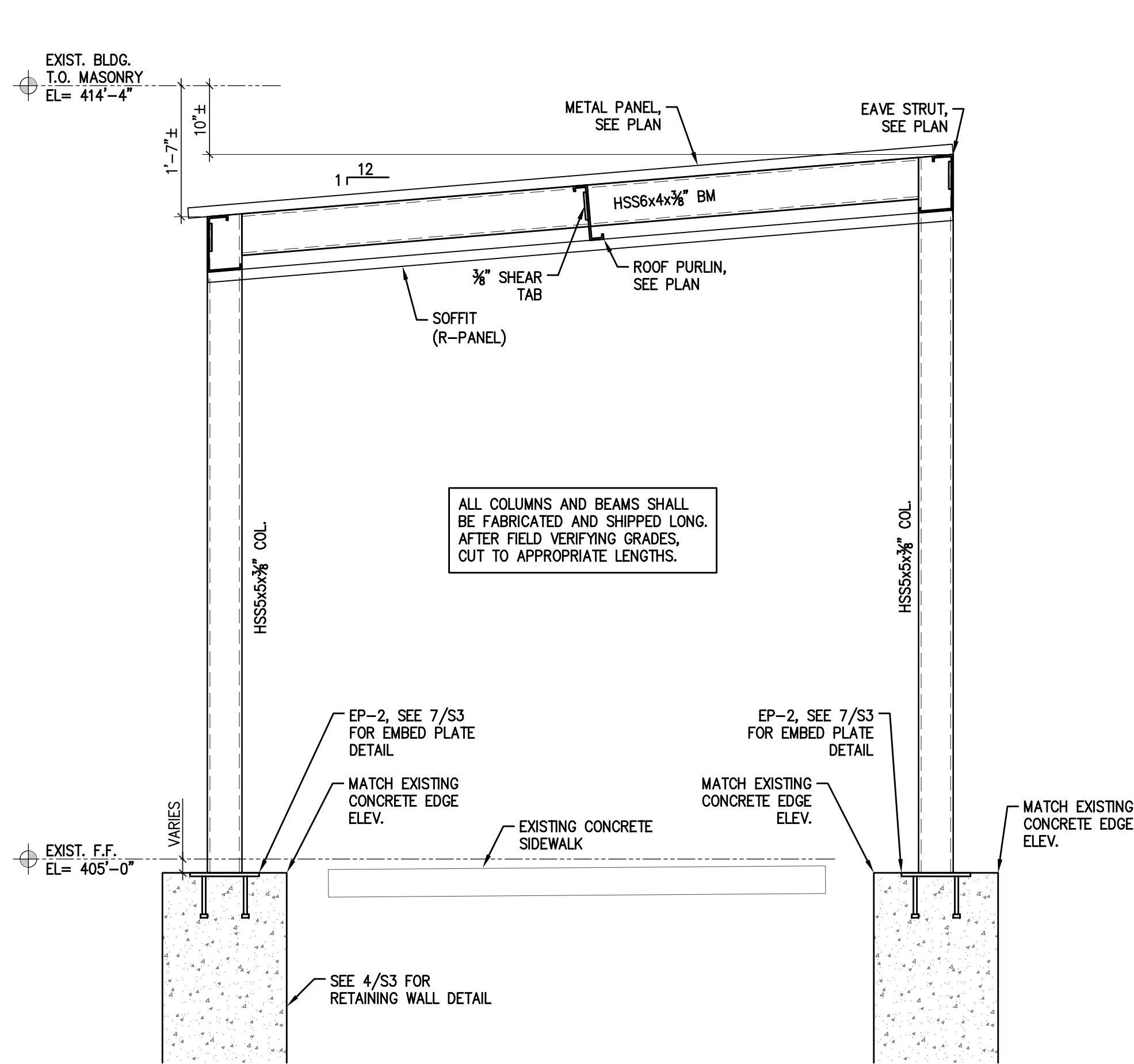
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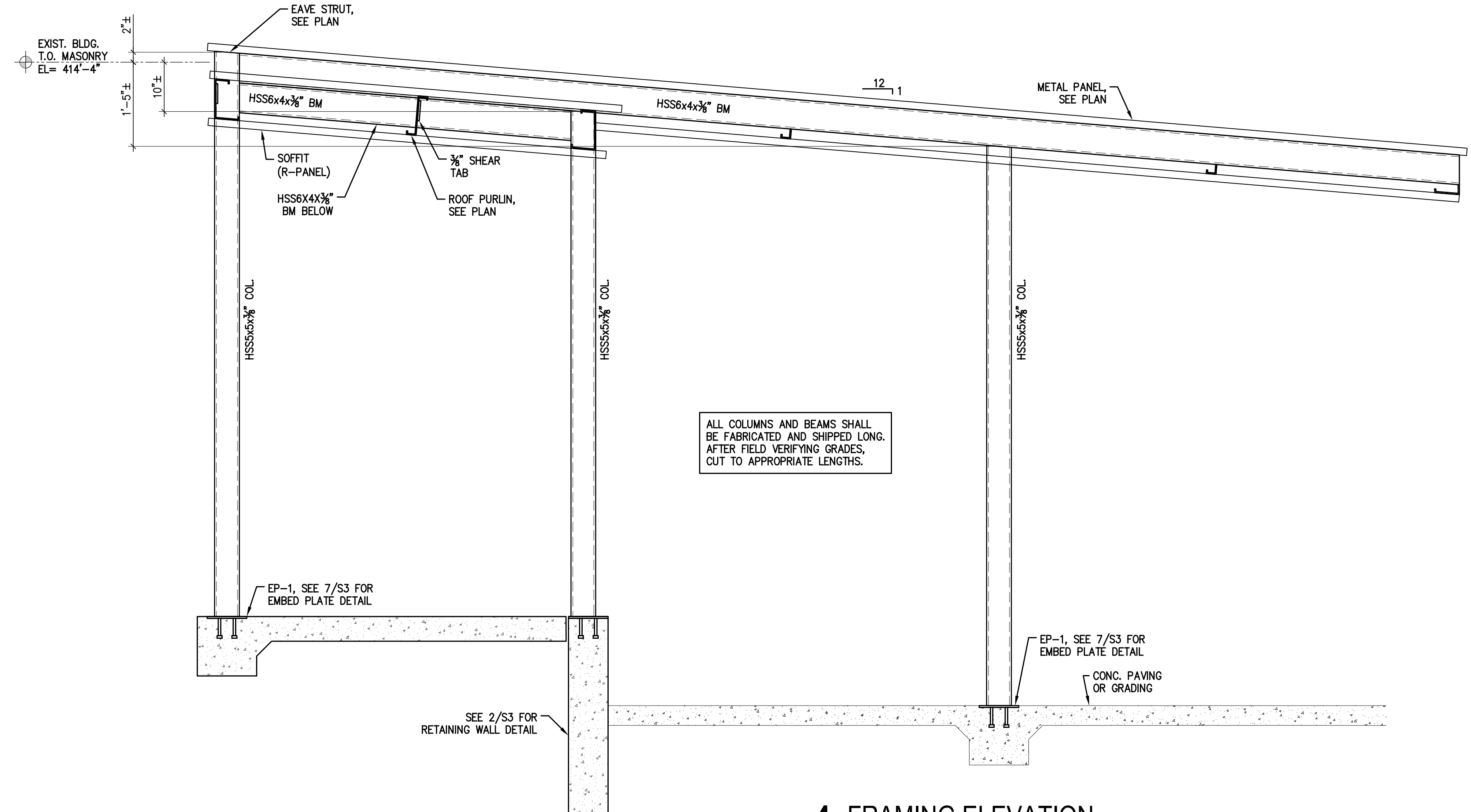
**1 FRAMING ELEVATION**  
Scale: 3/4" = 1'-0"



**2 FRAMING ELEVATION**  
Scale: 3/4" = 1'-0"

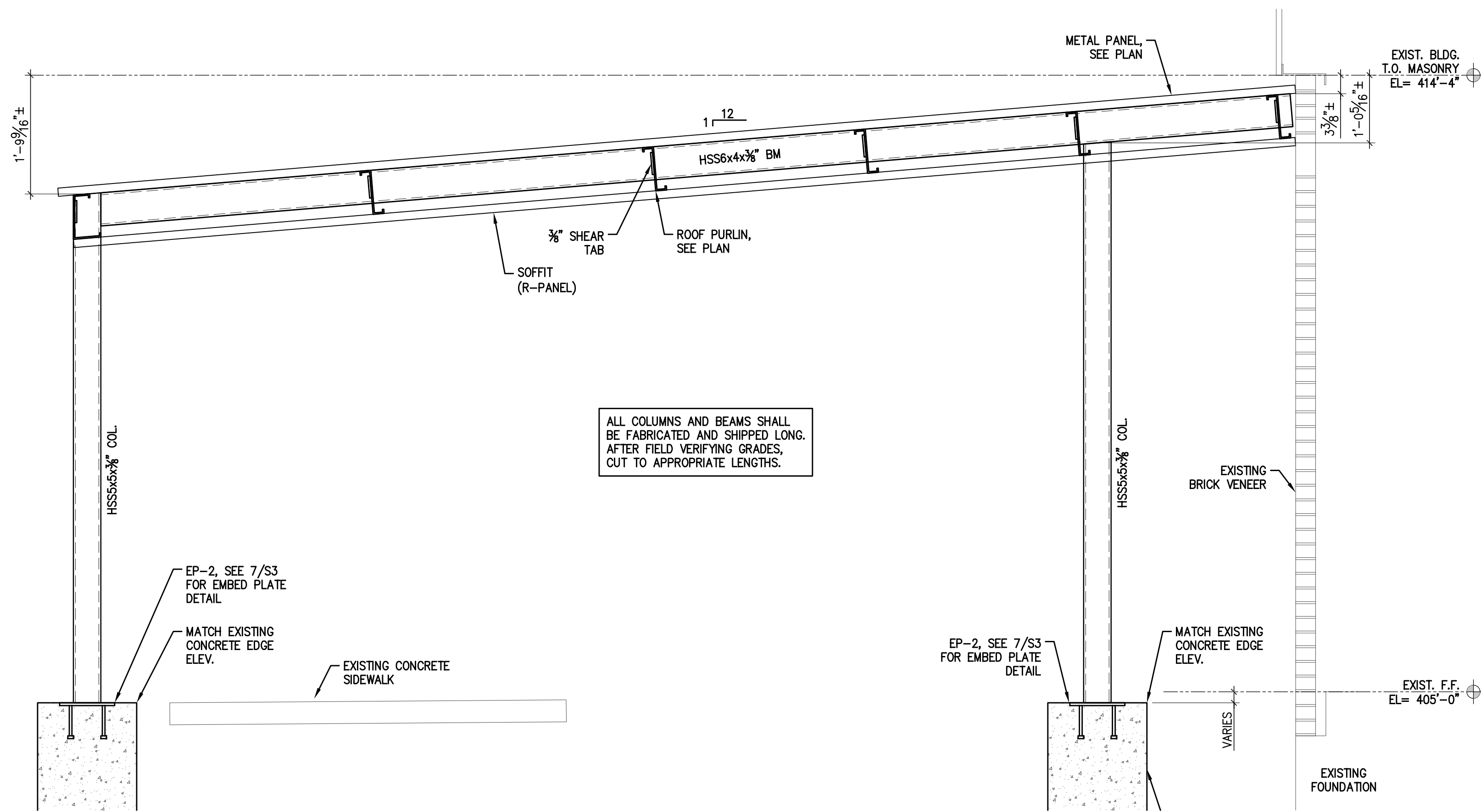


**3 FRAMING ELEVATION**  
Scale: 3/4" = 1'-0"

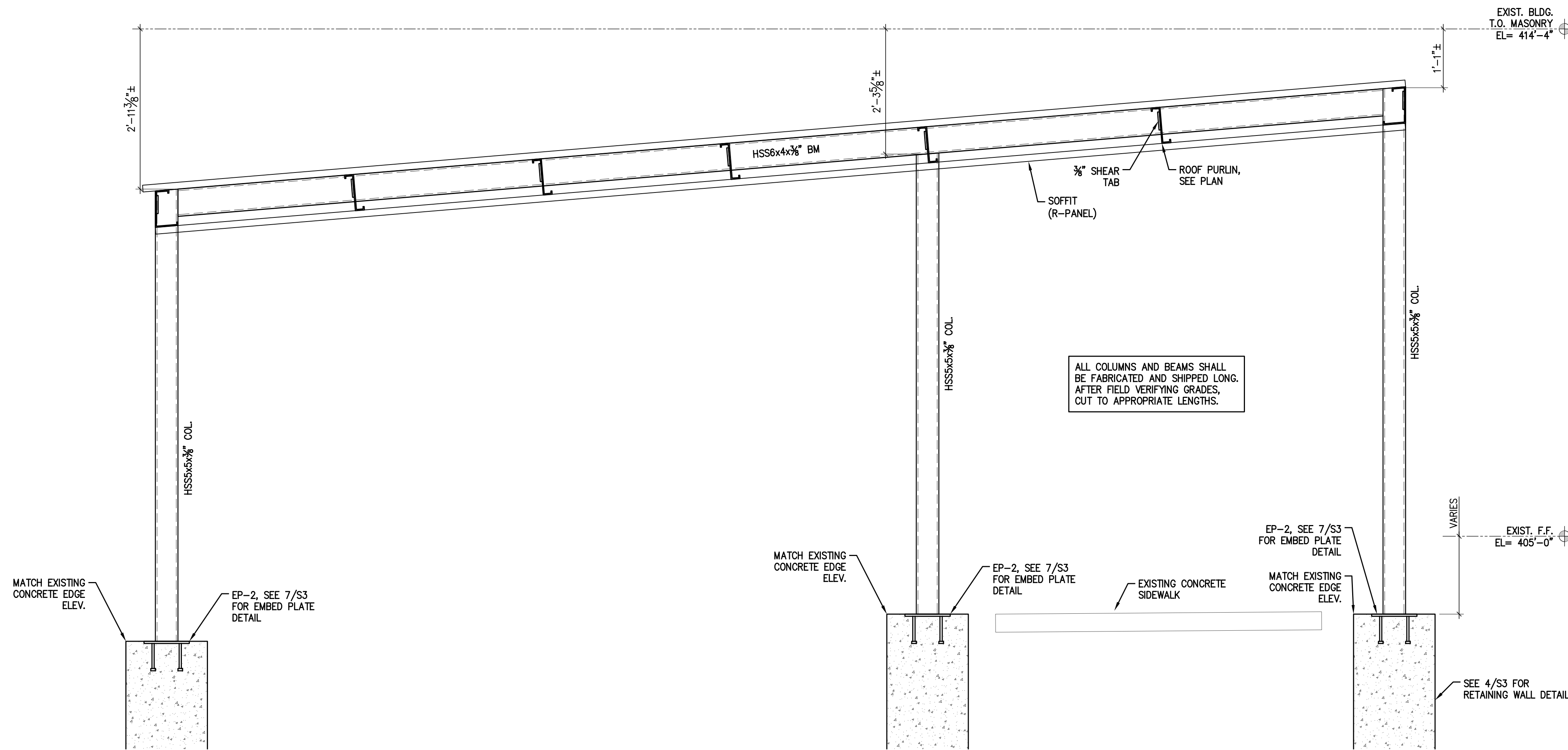


**4 FRAMING ELEVATION**  
Scale: 3/4" = 1'-0"

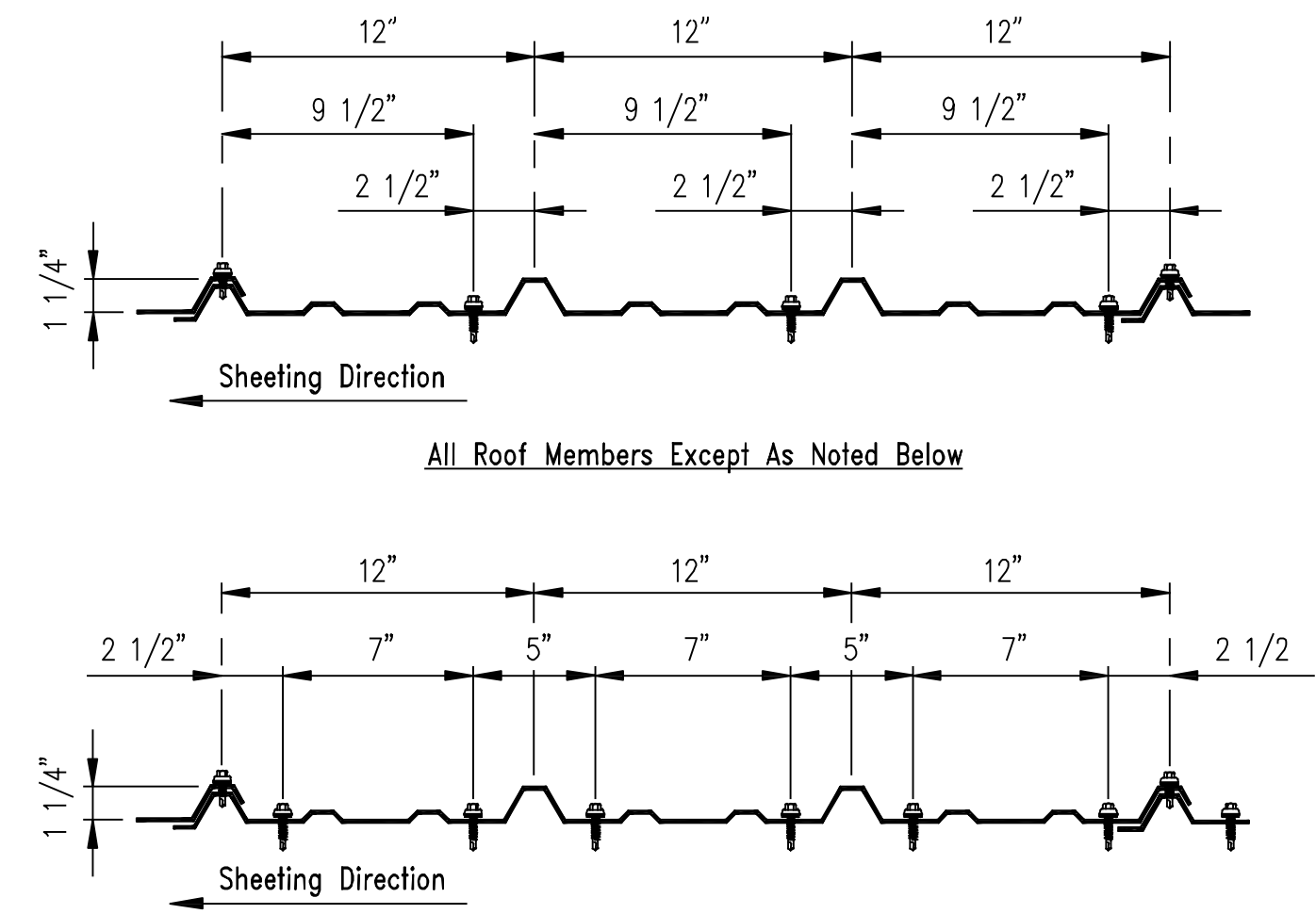
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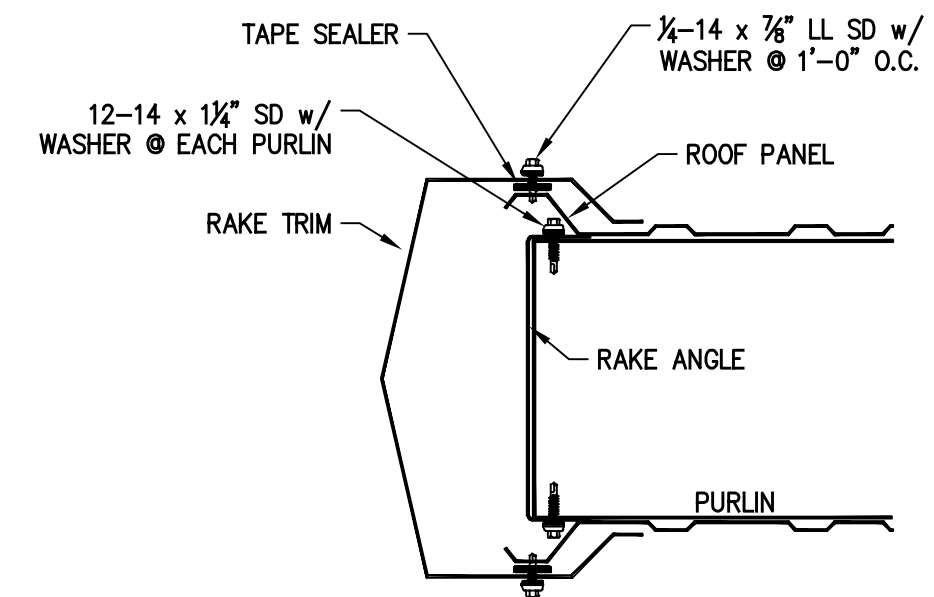
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Scale: 3/4" = 1'-0"



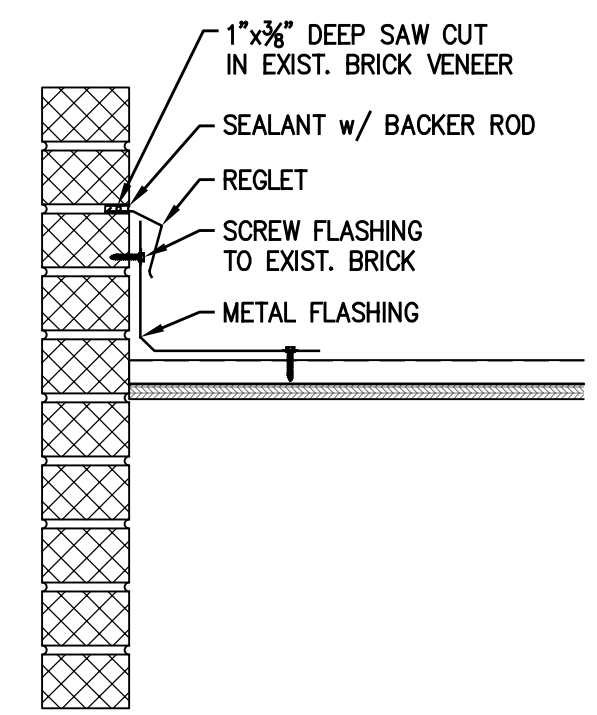
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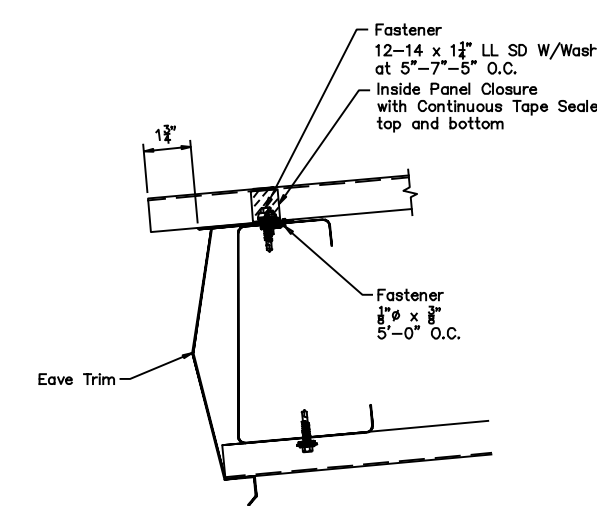
At Eave Strut, Panel End Lap, and Peak Purlin  
**PANEL FASTENER LOCATION**



**TRIM AT ENDWALL DETAIL**



**TRIM AT BRICK VENEER**



**TRIM AT EAVE**